

Chapter 03 - Superstructure

SECTION 05

**STRUCTURAL
STEEL
(SUP-SS)**

Chapter 03 - Superstructure

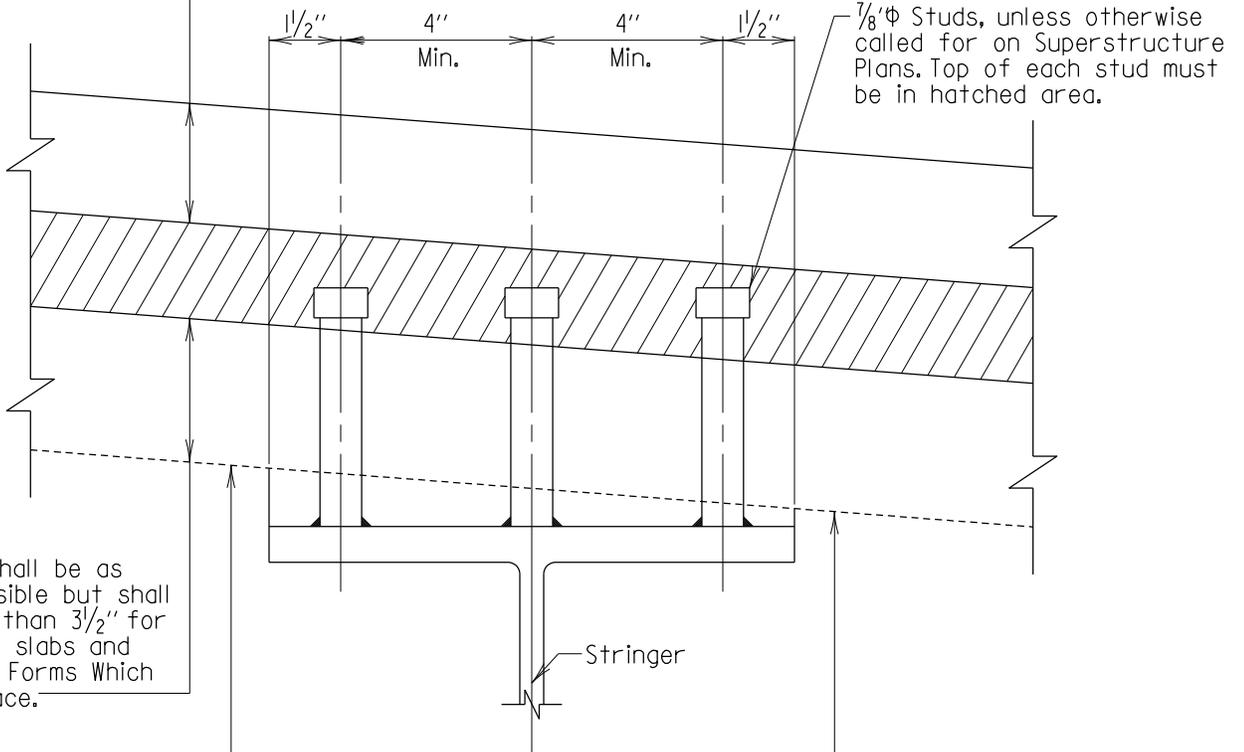
Section 05 – Structural Steel

SUB-SECTION 01

GENERAL

(SUP-SS(GEN))

In no case shall cover be less than 2 1/2" (Typ.).



Embedment shall be as deep as possible but shall not be less than 3 1/2" for wood formed slabs and 3" for Steel Forms Which Remain in Place.

Bottom of slab for wood formed slabs or top of Steel Forms Which Remain in Place.

ELEVATION

Scale: None

Notes:

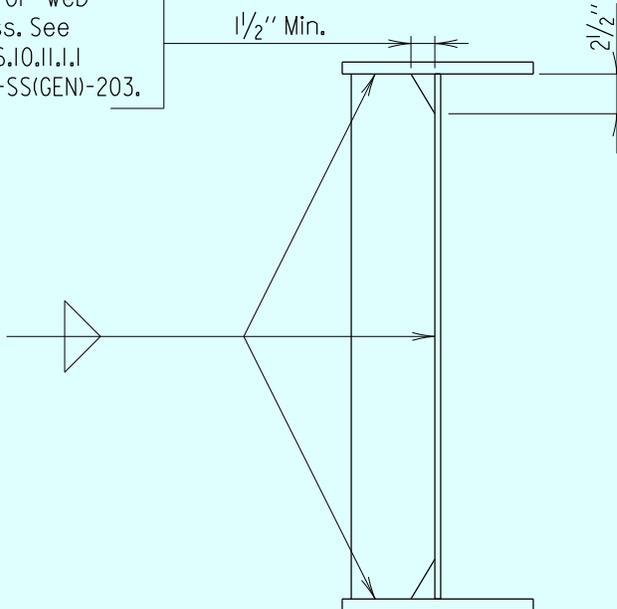
1. For number of studs per row, and longitudinal spacing of rows see pertinent Superstructure sheets.
2. For flange widths less than 11", only two rows of studs are to be used.
3. Steel Forms Which Remain in Place not shown.

APPROVAL
<i>L.S. Friedman</i> DIRECTOR OFFICE OF STRUCTURES
DATE: 09/08/1976
VERSION
1.0

STATE OF MARYLAND DEPARTMENT OF TRANSPORTATION STATE HIGHWAY ADMINISTRATION OFFICE OF STRUCTURES
STEEL STUD SHEAR DEVELOPER EMBEDMENT DETAIL
DETAIL NO. SUP-SS(GEN)-101
SHEET <u>1</u> OF <u>1</u>

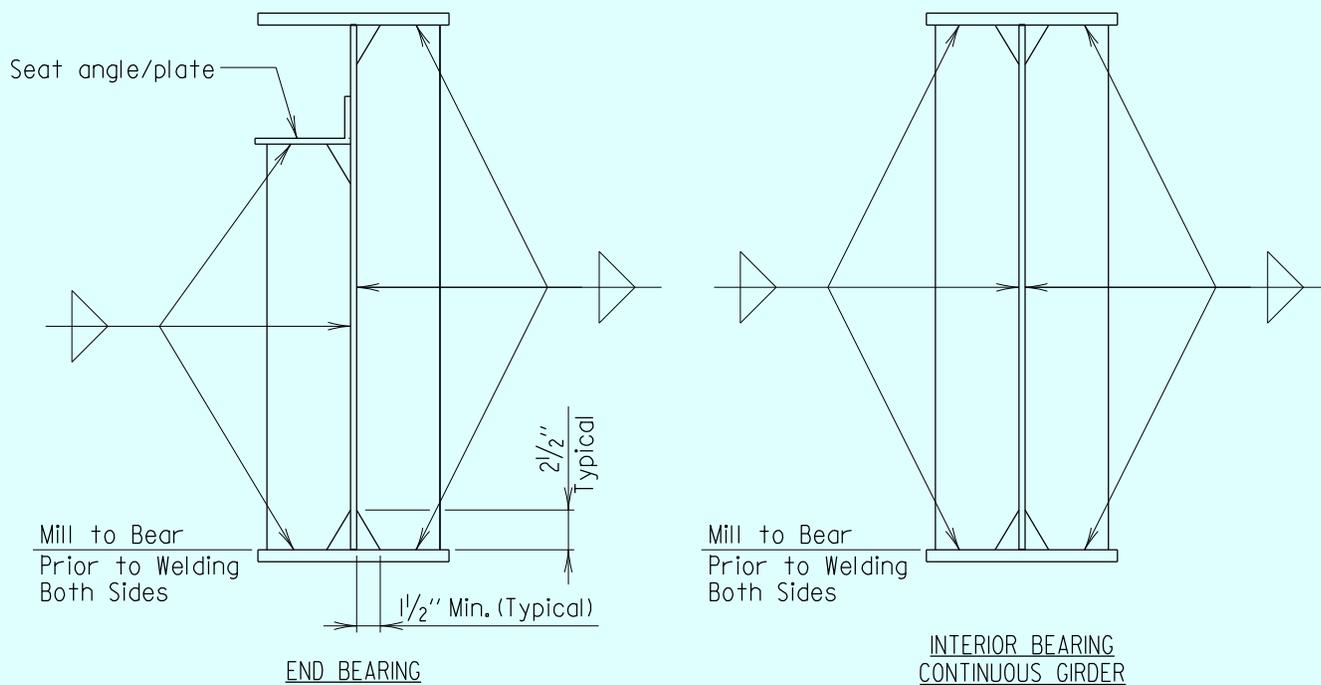
SUPERSTRUCTURE STEEL

Adjust for web thickness. See AASHTO 6.10.11.1.1 and SUP-SS(GEN)-203.



INTERMEDIATE STIFFENERS - ANGLE CLIPS

Scale: 1" = 1'-0"



BEARING STIFFENERS - ANGLE CLIPS

Scale: 1" = 1'-0"

Notes:

1. Minimum stiffener thickness $\frac{1}{2}$ ".
2. On exterior girders place all intermediate stiffeners on inside of girder.
3. When longitudinal stiffeners are required, place all longitudinal stiffeners on one side of web, place transverse stiffeners on opposite side.
4. Minimum fillet weld is $\frac{5}{16}$ ".

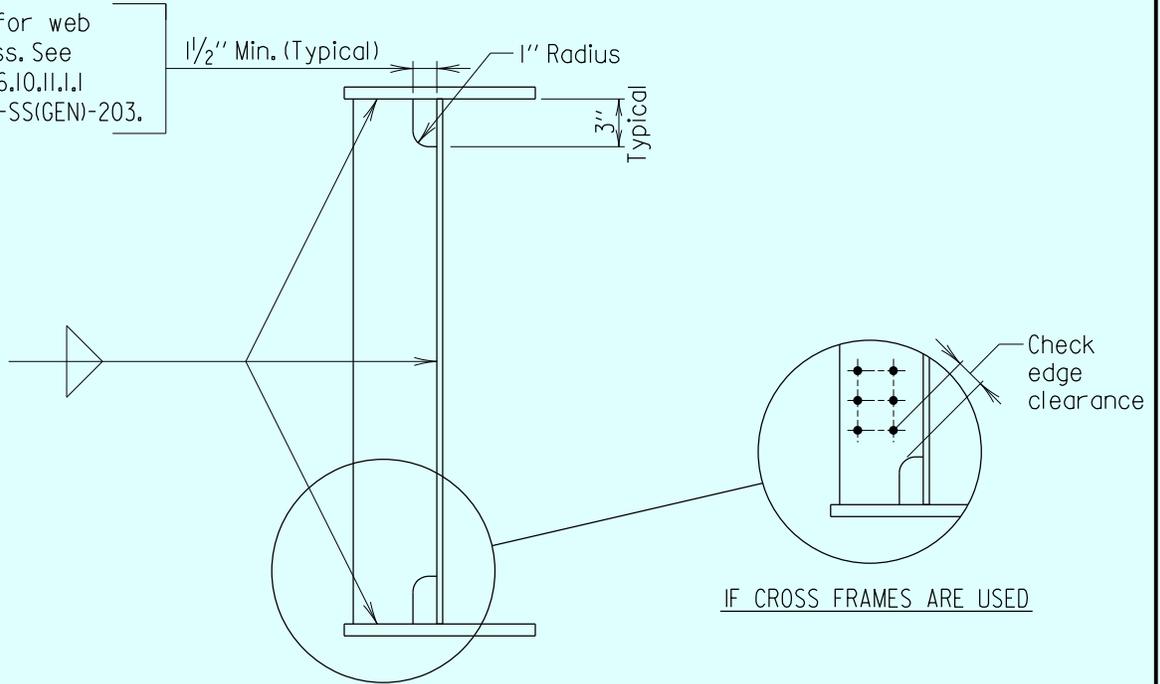
* FOR OFFICE USE ONLY *

APPROVAL
<i>E.S. Friedman</i> DIRECTOR OFFICE OF STRUCTURES
DATE: 10/09/2007
VERSION
1.0

STATE OF MARYLAND DEPARTMENT OF TRANSPORTATION STATE HIGHWAY ADMINISTRATION OFFICE OF STRUCTURES	
STIFFENER ATTACHMENT DETAILS FOR STEEL GIRDERS ANGLE CLIP	
DETAIL NO. SUP-SS(GEN)-201	SHEET <u>1</u> OF <u>2</u>

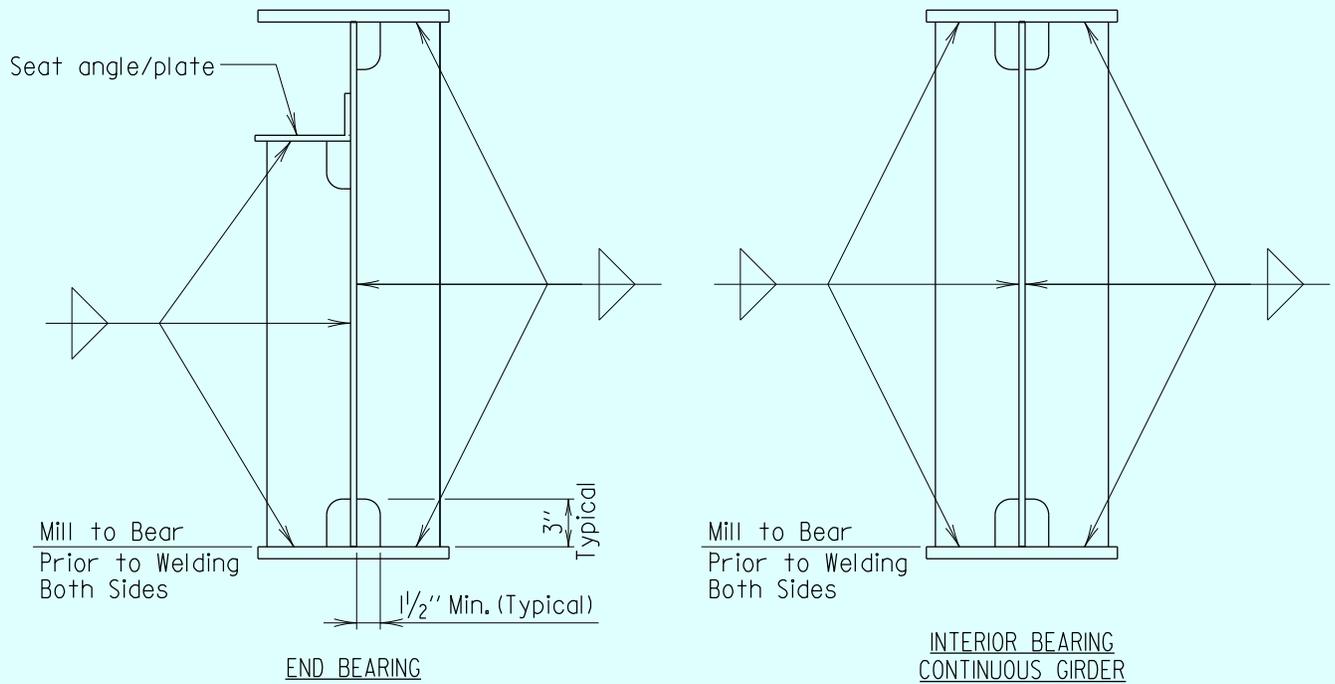
SUPER-STRUCT. STEEL

Adjust for web thickness. See AASHTO 6.10.11.1.1 and SUP-SS(GEN)-203.



INTERMEDIATE STIFFENERS - RADIUS CLIPS

Scale: 1" = 1'-0"



BEARING STIFFENERS - RADIUS CLIPS

Scale: 1" = 1'-0"

Notes:

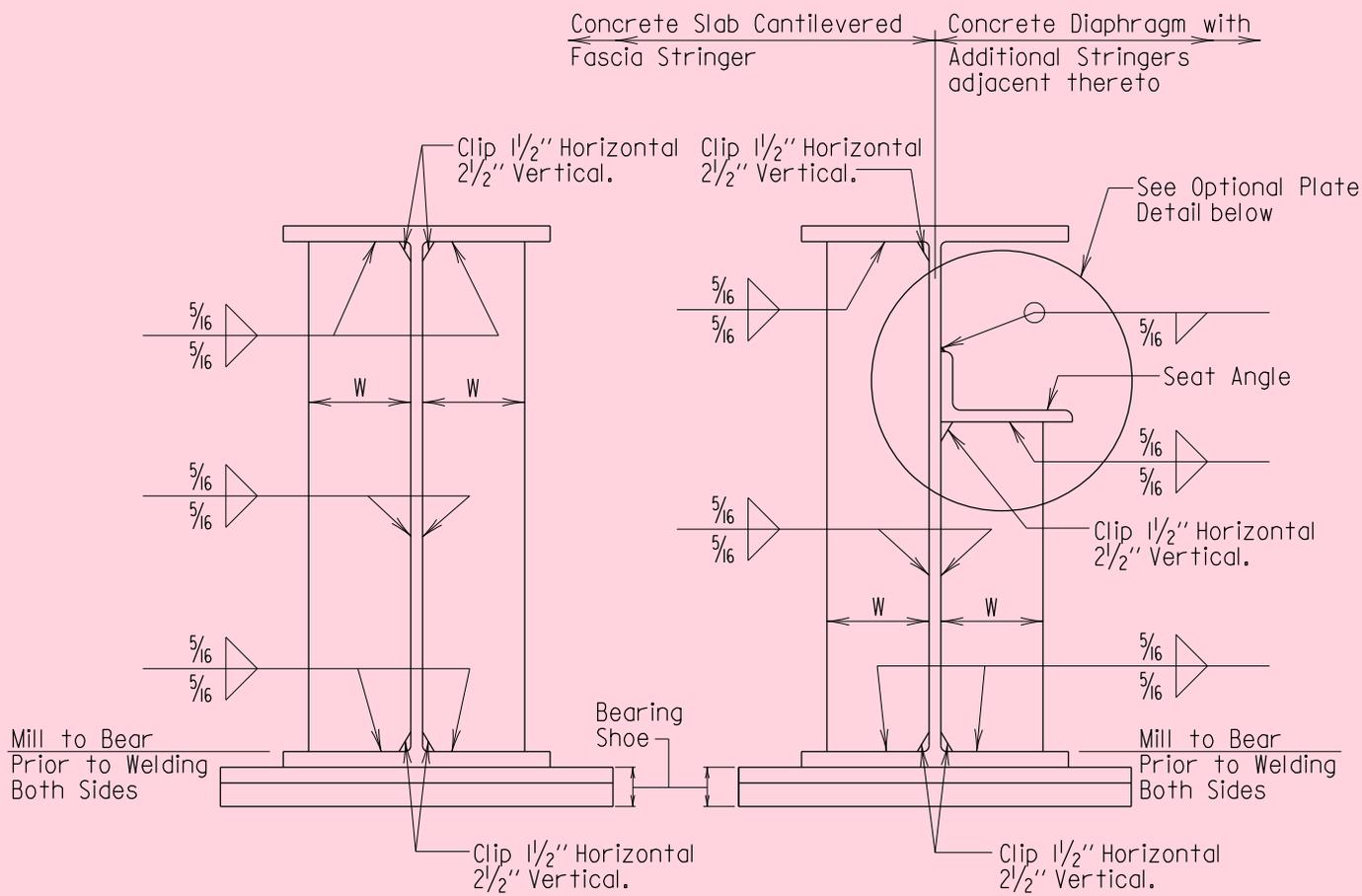
1. Minimum stiffener thickness $\frac{1}{2}$ ".
2. On exterior girders place all intermediate stiffeners on inside of girder.
3. When longitudinal stiffeners are required, place all longitudinal stiffeners on one side of web, place transverse stiffeners on opposite side.
4. Minimum fillet weld is $\frac{5}{16}$ ".

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APPROVAL
<i>E.S. Friedman</i> DIRECTOR OFFICE OF STRUCTURES
DATE: 10/09/2007
VERSION
1.0

STATE OF MARYLAND DEPARTMENT OF TRANSPORTATION STATE HIGHWAY ADMINISTRATION OFFICE OF STRUCTURES
STIFFENER ATTACHMENT DETAILS WITH OPTIONAL RADIUS CLIPS FOR STEEL GIRDERS
DETAIL NO. SUP-SS(GEN)-201
SHEET 2 OF 2

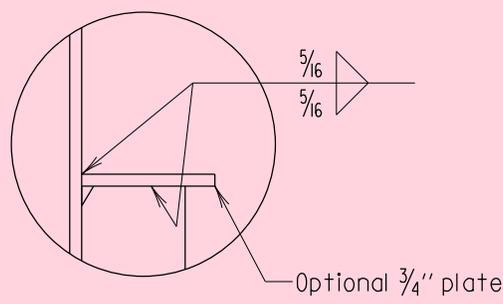
SUPERSTRUCTURE STEEL



AT PIERS
 (WHERE STRINGER IS
 CONTINUOUS OVER SUPPORT)
 Scale: 1/2" = 1'-0"

AT PIERS
 (WHERE STRINGER IS NOT
 CONTINUOUS OVER SUPPORT) AND
AT ABUTMENTS
 Scale: 1/2" = 1'-0"

Location	W= Stiffener Width	Stiffener Thickness
Abutment		
Pier		
Pier		
Pier		
Abutment		



OPTIONAL SEAT PLATE DETAIL

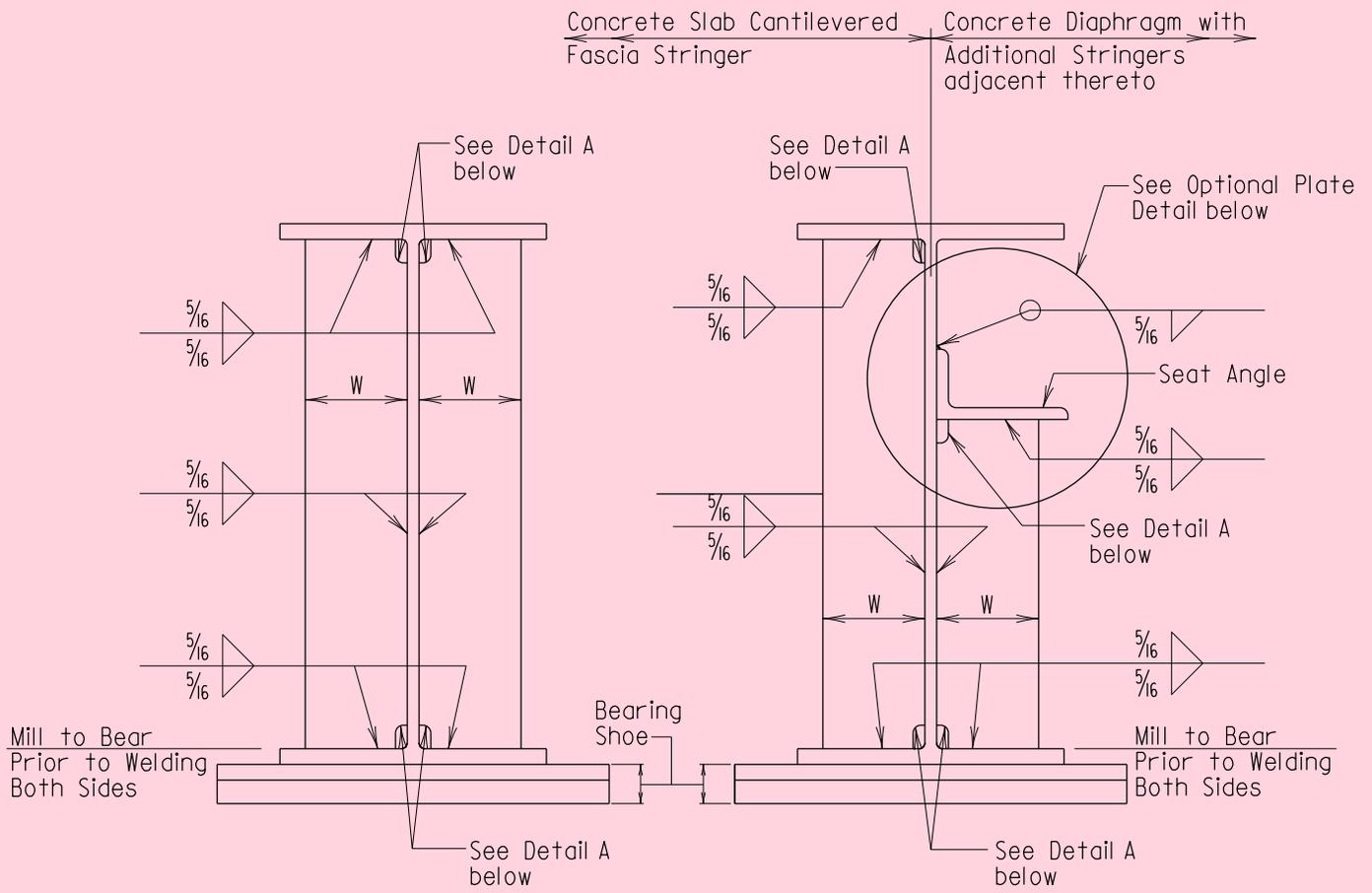
APPROVAL
<i>Ben C. [Signature]</i> DIRECTOR OFFICE OF STRUCTURES
DATE: 04/03/2018
VERSION
1.01

STATE OF MARYLAND
 DEPARTMENT OF TRANSPORTATION
 STATE HIGHWAY ADMINISTRATION
 OFFICE OF STRUCTURES

**BEARING STIFFENERS FOR ROLLED STEEL BEAMS
 ANGLE CLIP**

DETAIL NO. SUP-SS(GEN)-202 SHEET 1 OF 2

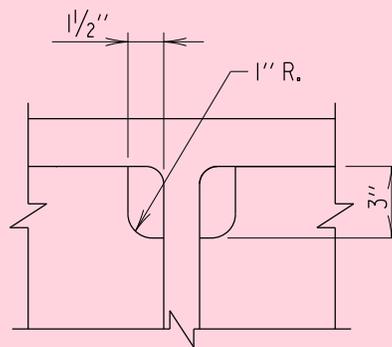
SUPERSTRUCTURE STEEL



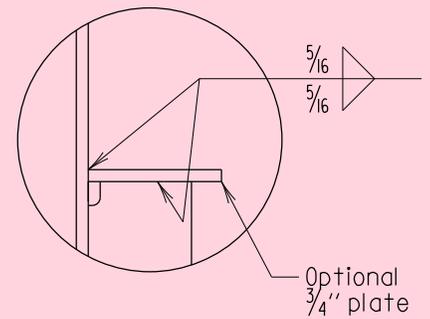
AT PIERS
(WHERE STRINGER IS
CONTINUOUS OVER SUPPORT)
 Scale: 1/2" = 1'-0"

AT PIERS
(WHERE STRINGER IS NOT
CONTINUOUS OVER SUPPORT) AND
AT ABUTMENTS
 Scale: 1/2" = 1'-0"

Location	W= Stiffener Width	Stiffener Thickness
Abutment		
Pier		
Pier		
Pier		
Abutment		



DETAIL A
 Scale: 1/2" = 1'-0"



OPTIONAL SEAT PLATE DETAIL

APPROVAL
<i>[Signature]</i> DIRECTOR OFFICE OF STRUCTURES
DATE: 04/03/2018
VERSION
1.01

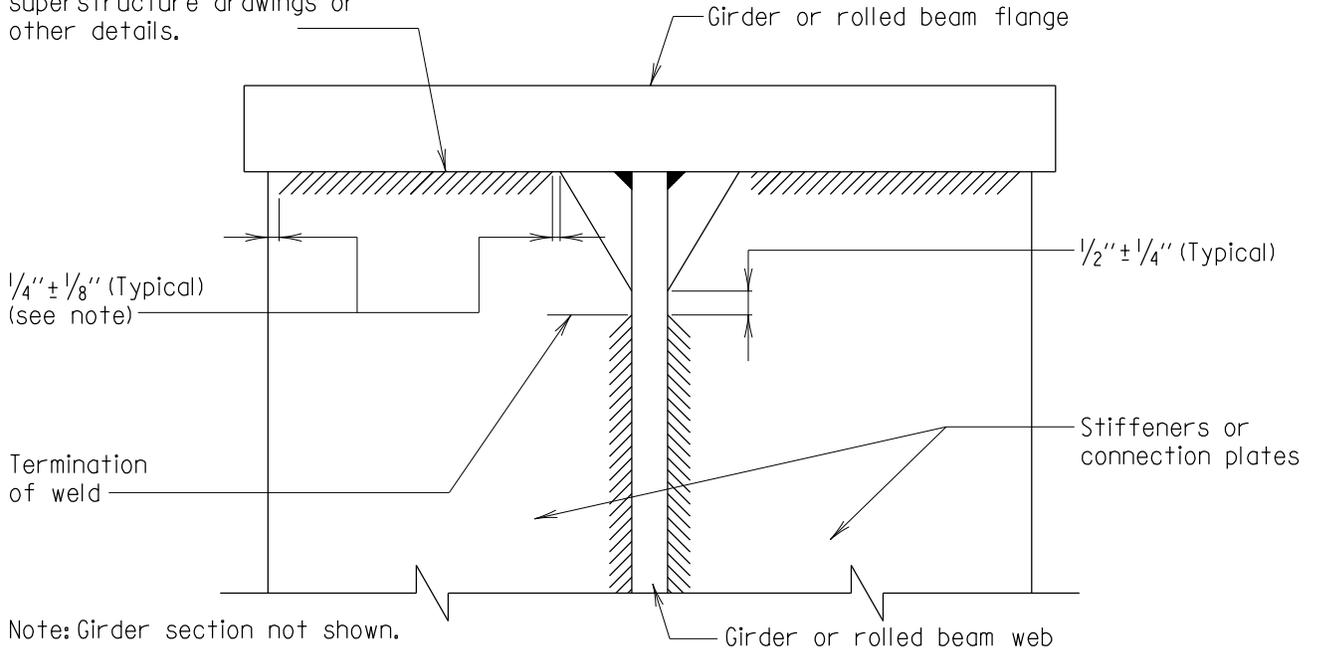
STATE OF MARYLAND
 DEPARTMENT OF TRANSPORTATION
 STATE HIGHWAY ADMINISTRATION
 OFFICE OF STRUCTURES

**BEARING STIFFENERS FOR ROLLED STEEL BEAMS
 OPTIONAL RADIUS CLIP**

DETAIL NO. SUP-SS(GEN)-202 SHEET 2 OF 2

SUPERSTRUCTURE STEEL

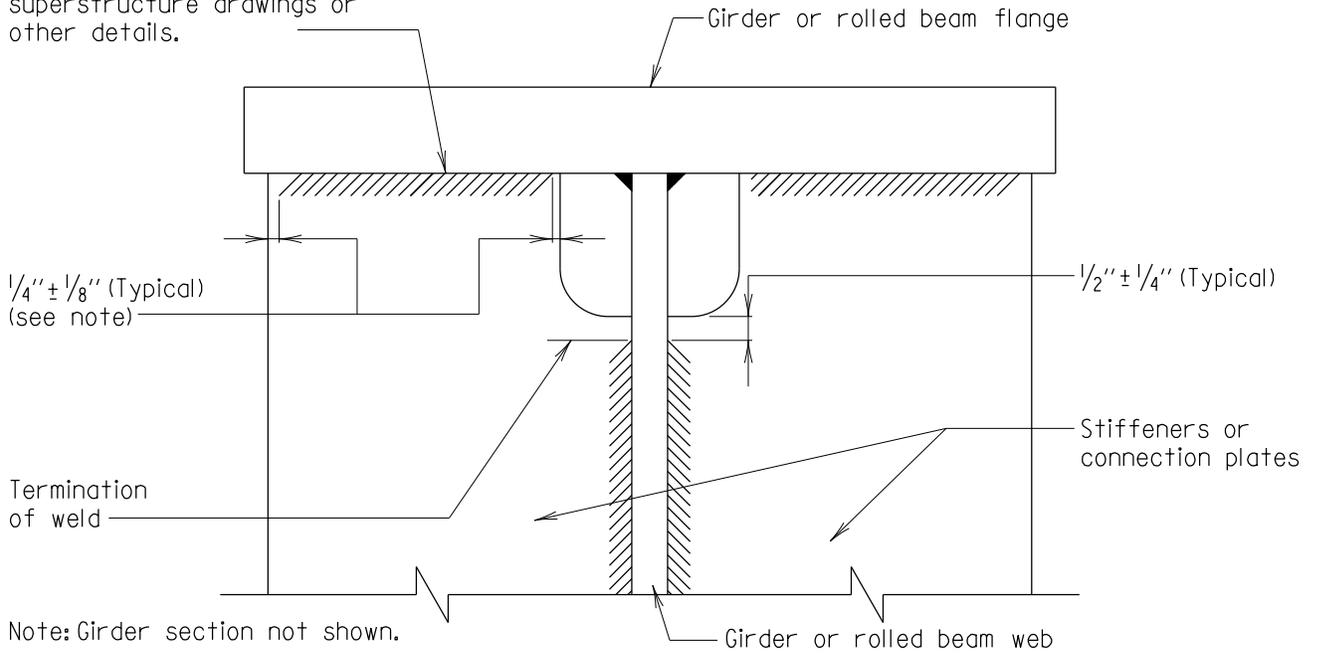
Weld only where indicated on superstructure drawings or other details.



SECTION - ANGLE CLIP

Scale: 3" = 1'-0"

Weld only where indicated on superstructure drawings or other details.



SECTION - OPTIONAL RADIUS CLIP

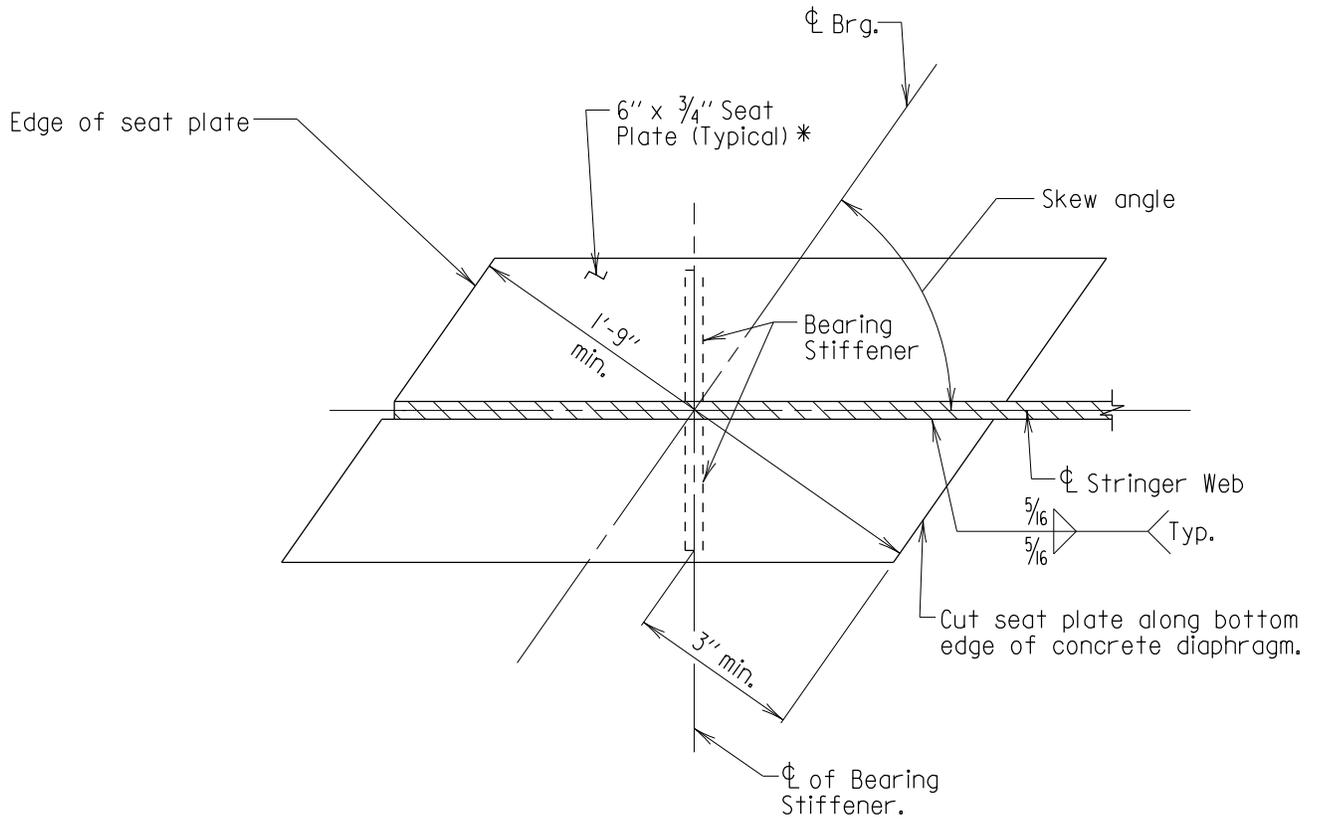
Scale: 3" = 1'-0"

Notes:

1. For all stiffeners (intermediate or bearing) top and bottom, including connection plate for channel diaphragms for all girders and rolled beams.
2. Welding to flange as per this detail will only be required where plans or other detail sheets indicate stiffener is extended and welded to flange.

APPROVAL
<i>E.S. Freeman</i> DIRECTOR OFFICE OF STRUCTURES
DATE: 10/01/2003
VERSION
1.0

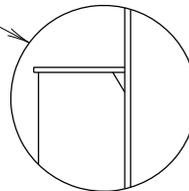
STATE OF MARYLAND DEPARTMENT OF TRANSPORTATION STATE HIGHWAY ADMINISTRATION OFFICE OF STRUCTURES
STIFFENER AND STRINGER CONNECTION PLATE WELD TERMINATION DETAIL
DETAIL NO. SUP-SS(GEN)-203
SHEET <u> 1 </u> OF <u> 1 </u>



PLAN VIEW OF STRINGER BELOW TOP FLANGE

Scale: 1 1/2" = 1'-0"

* Seat plate width shall be increased as necessary so that plate exceeds stiffener width by at least 1/2".

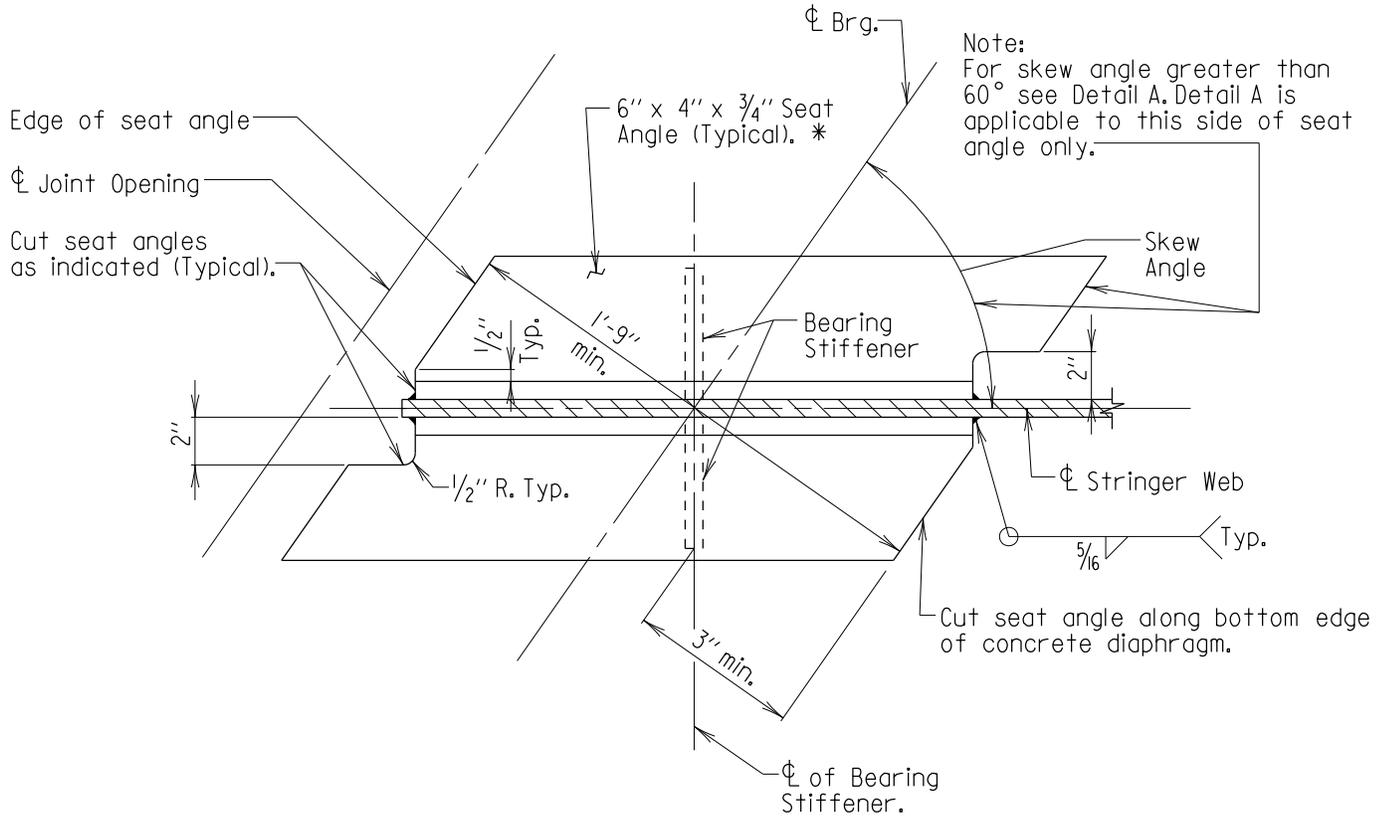


Notes:

1. Contractor has the option of using seat plates or seat angles, only one type shall be used per bridge.
2. Concrete diaphragm not shown.

APPROVAL
<i>L.S. Friedman</i> DIRECTOR OFFICE OF STRUCTURES
DATE: 03/07/2013
VERSION
1.0

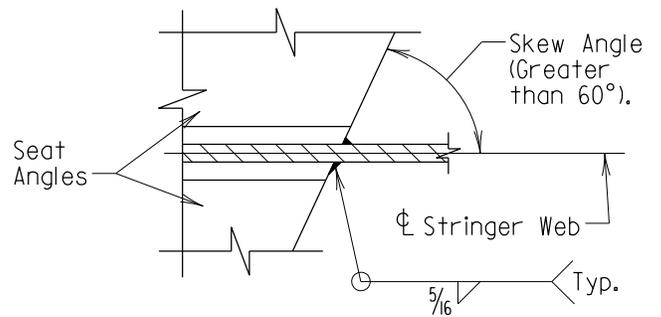
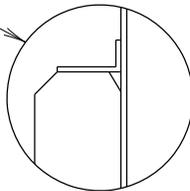
STATE OF MARYLAND DEPARTMENT OF TRANSPORTATION STATE HIGHWAY ADMINISTRATION OFFICE OF STRUCTURES
STEEL SEAT PLATES FOR SKEWED CONCRETE DIAPHRAGMS
DETAIL NO. SUP-SS(GEN)-204
SHEET <u>1</u> OF <u>2</u>



PLAN VIEW OF STRINGER BELOW TOP FLANGE

Scale: 1/2" = 1'-0"

* Longest leg of angle shall be increased as necessary so that angle exceeds stiffener width by at least 1/2". If angle size is not available to satisfy this requirement, stiffener shall be tapered at end to meet this requirement.



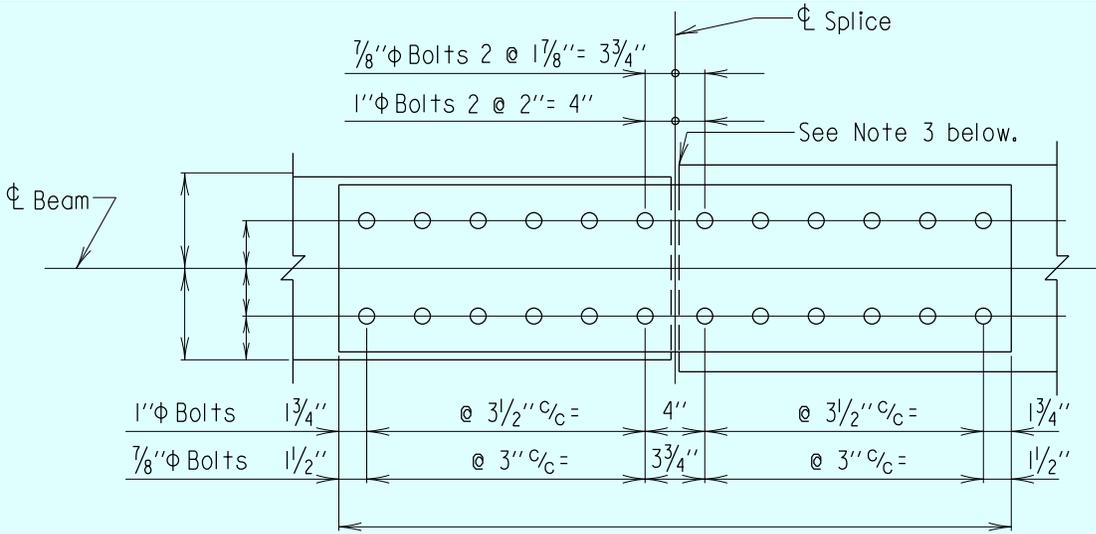
DETAIL A

Scale: 1/2" = 1'-0"

Note:
Concrete diaphragm not shown.

APPROVAL
<i>L.S. Freeman</i> DIRECTOR OFFICE OF STRUCTURES
DATE: 03/07/2013
VERSION
1.0

STATE OF MARYLAND DEPARTMENT OF TRANSPORTATION STATE HIGHWAY ADMINISTRATION OFFICE OF STRUCTURES	
STEEL SEAT ANGLES FOR SKEWED CONCRETE DIAPHRAGMS	
DETAIL NO. SUP-SS(GEN)-204	SHEET 2 OF 2

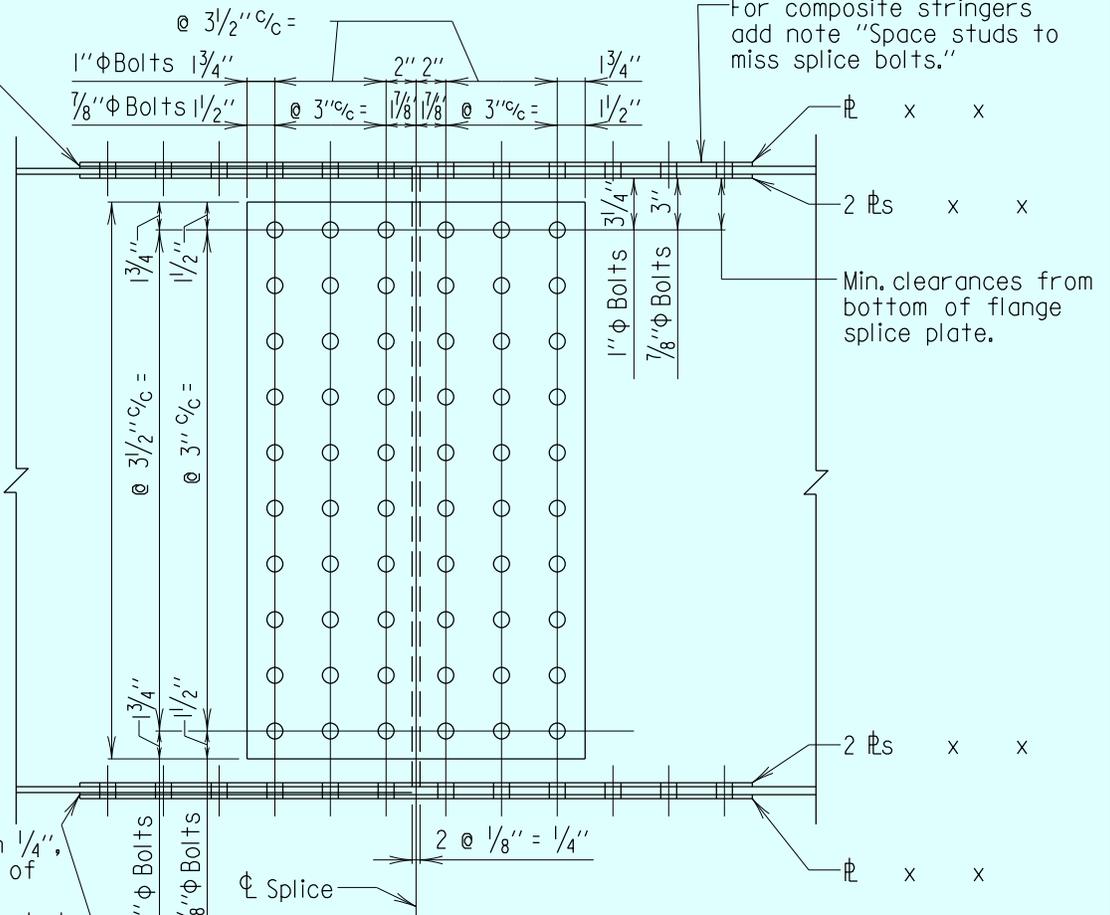


PLAN

Scale: 1" = 1'-0"

Filler ϕ
(where required, $\frac{1}{16}$ "
minimum thickness).

For composite stringers
add note "Space studs to
miss splice bolts."



ELEVATION

Scale: 1" = 1'-0"

Filler ϕ
(where required, $\frac{1}{16}$ "
minimum thickness).
If filler is greater than $\frac{1}{4}$ ",
the need for extension of
filler and/or additional
fasteners must be evaluated
as per AASHTO requirements.

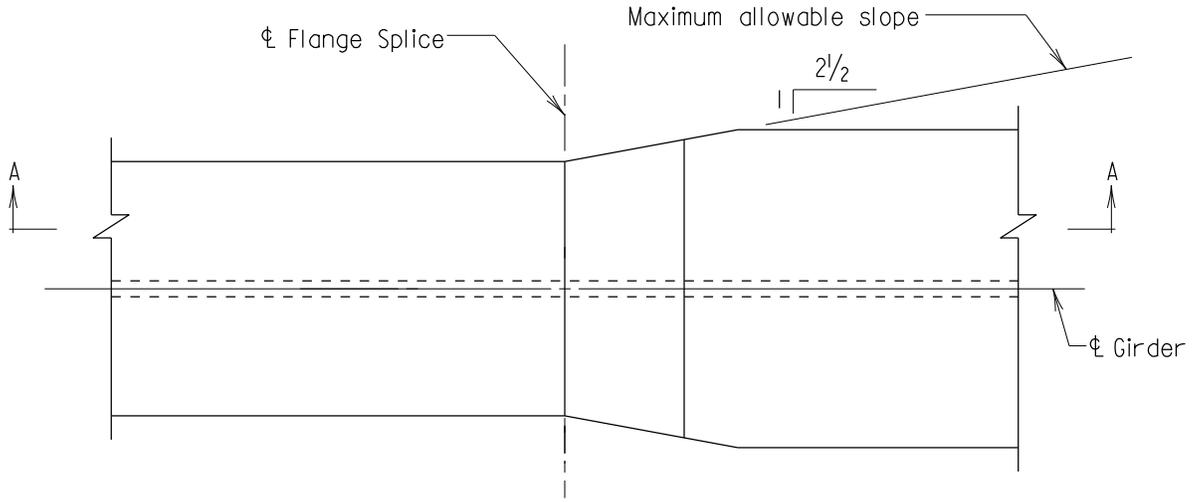
- Notes:
- All bolts to be $\frac{7}{8}$ " ϕ High Strength Bolts ($\frac{15}{16}$ " ϕ open holes), ASTM A 325 unless 1" ϕ High Strength Bolts are required by design ($\frac{11}{16}$ " open holes). Bolts to be used when weathering structural steel is called for shall be ASTM A 325, Type 3.
 - All splice plates to be a minimum $\frac{1}{2}$ " thick.
 - If flange widths of adjacent stringers vary more than 2", then larger flange shall be tapered to smaller flange width in a distance of $\frac{1}{2}$ length of cover plate. This only applies to bottom flange.
 - Bolts not shown in splice.
 - Bolt heads shall be on the exterior face of the fascia stringer and the bottom of bottom flange.

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APPROVAL
<i>E.S. Friedman</i> DIRECTOR OFFICE OF STRUCTURES
DATE: 10/09/2007
VERSION
1.0

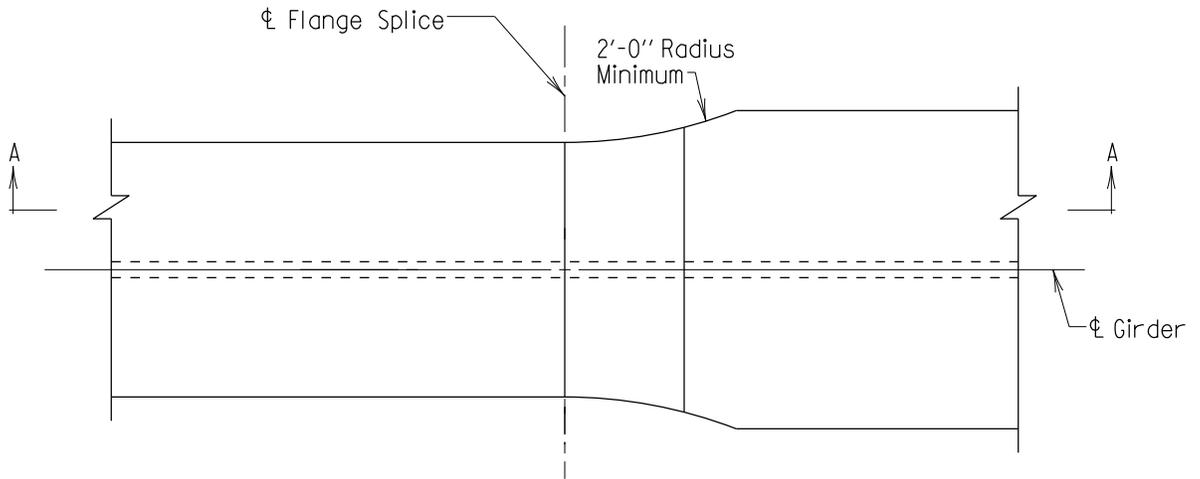
STATE OF MARYLAND DEPARTMENT OF TRANSPORTATION STATE HIGHWAY ADMINISTRATION OFFICE OF STRUCTURES
TYPICAL STEEL STRINGER SPlice DETAIL
DETAIL NO. SUP-SS(GEN)-301
SHEET <u> X </u> OF <u> X </u>

SUPERSTRUCTURE STEEL



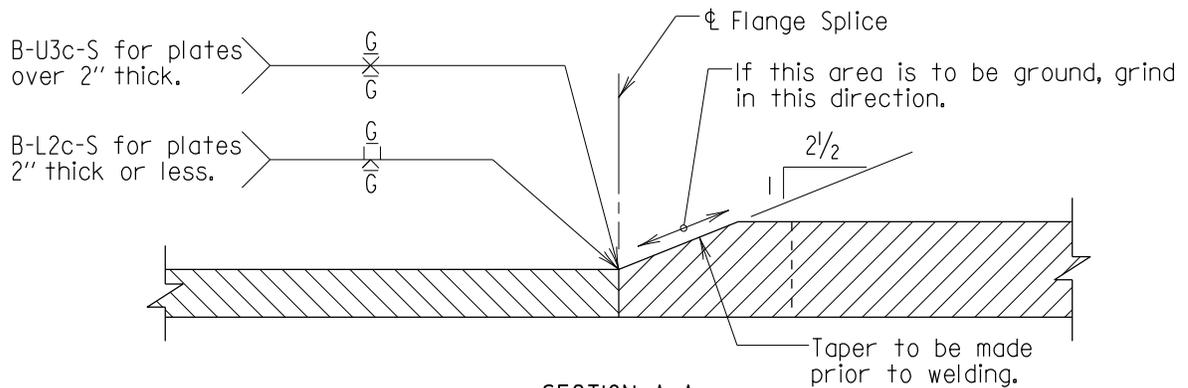
PLAN OF STRAIGHT TAPERED TRANSITION FLANGE SPLICE

Scale: None



PLAN OF RADIAL TRANSITION FLANGE SPLICE

Scale: None



SECTION A-A

Scale: None

Notes:

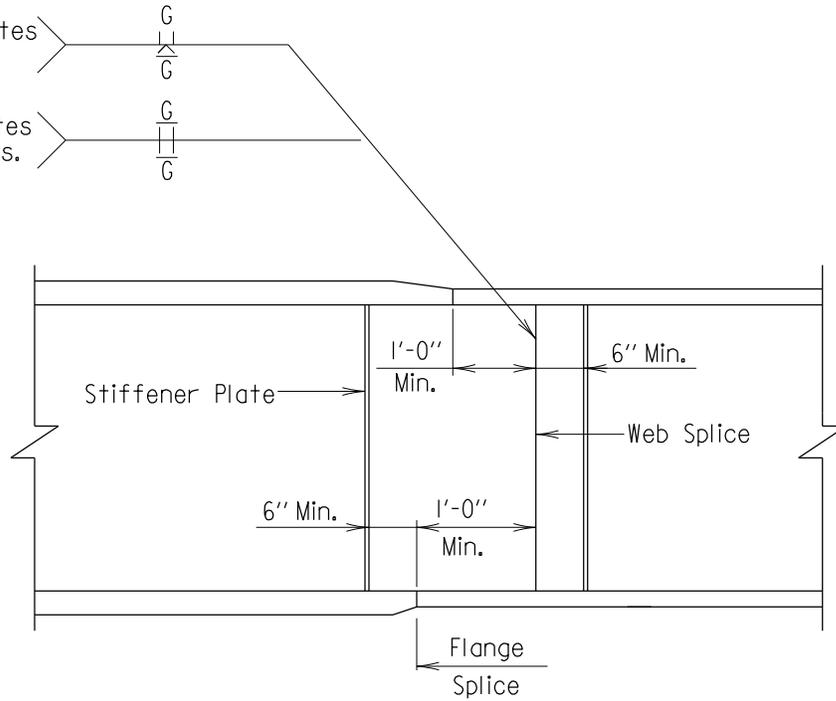
1. Butt welds of flange splice plates to be ground flush prior to attaching web plates.
2. Splice shown is for different width and different thickness flanges; if only one variation is present use pertinent portion of detail.
3. Fabricator may use either of the above transition details.

APPROVAL
<i>L.S. Freedom</i> DIRECTOR OFFICE OF STRUCTURES
DATE: 08/16/1992
VERSION
1.0

STATE OF MARYLAND DEPARTMENT OF TRANSPORTATION STATE HIGHWAY ADMINISTRATION OFFICE OF STRUCTURES
SHOP SPLICE DETAILS FOR PLATE GIRDER STRINGERS
DETAIL NO. SUP-SS(GEN)-401
SHEET <u>1</u> OF <u>2</u>

B-L2c-S for plates
over 1/2" thick.

B-L1a-S for plates
1/2" thick or less.



ELEVATION OF GIRDER

Scale: None

APPROVAL
<i>E.S. Friedman</i> DIRECTOR OFFICE OF STRUCTURES
DATE: 08/16/1992
VERSION
1.0

STATE OF MARYLAND DEPARTMENT OF TRANSPORTATION STATE HIGHWAY ADMINISTRATION OFFICE OF STRUCTURES	
SHOP SPLICE DETAILS FOR PLATE GIRDER STRINGERS	
DETAIL NO. SUP-SS(GEN)-401	SHEET 2 OF 2

SUPERSTRUCTURE STEEL

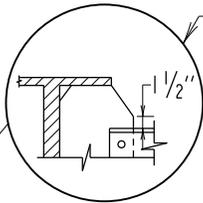
Chapter 03 - Superstructure

Section 05 – Structural Steel

SUB-SECTION 02

CROSS FRAMES
(SUP-SS(CF))

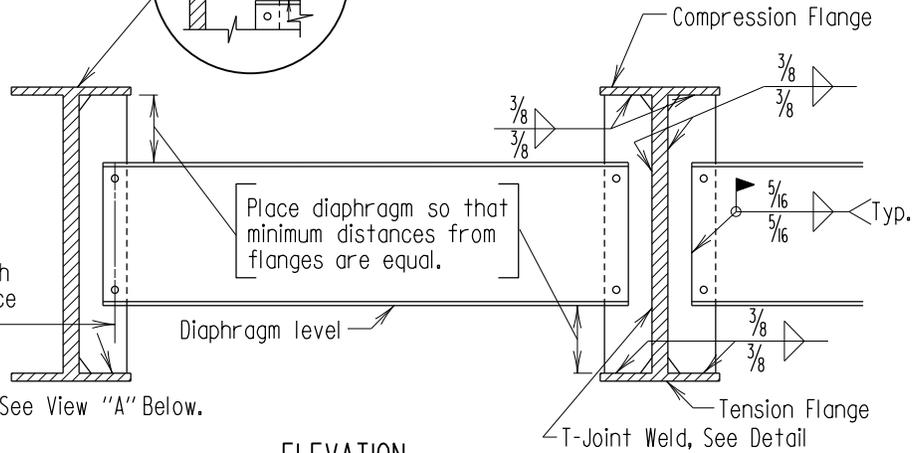
Note:
Angle clip shown;
optional radius clip
acceptable.



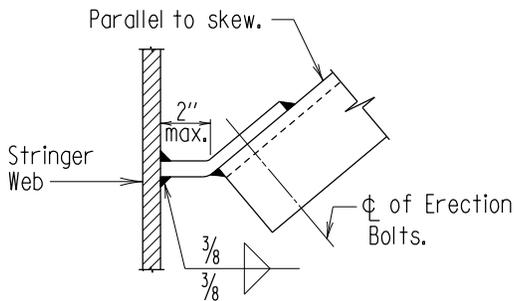
This detail to be used when
connection plate extend
beyond edge of flange.

2- $\frac{7}{8}$ " ϕ Erection Bolts in each
connection to remain in place
after welding.

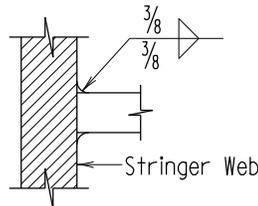
Note: See View "A" Below.



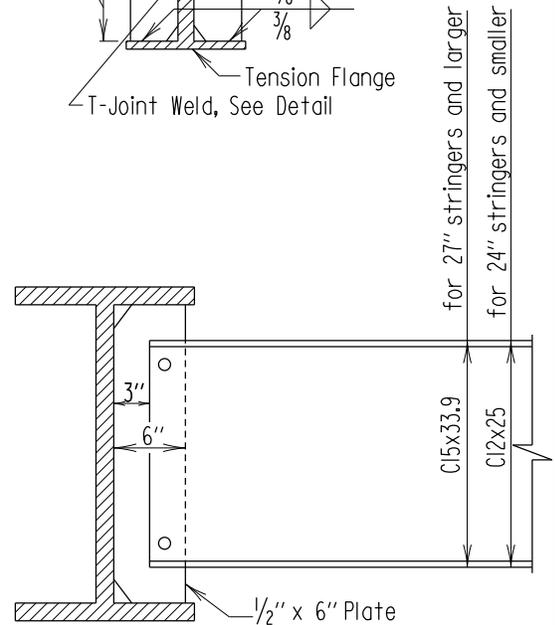
ELEVATION
Scale: None



T-JOINT WELD DETAIL FOR
SKEW ANGLE OVER 70° TO 90°
Scale: None



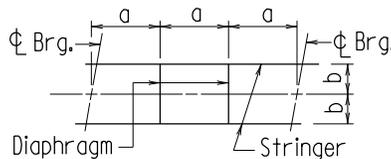
T-JOINT WELD DETAIL FOR
SKEW ANGLE 70° OR LESS
Scale: None



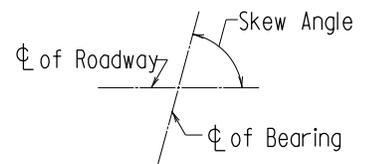
VIEW A
Scale: None

Notes:

1. *Slant lettering indicates note "For Office Use Only"*
2. Where the angle between the center line of roadway and the center line of bearing is 70° or less place diaphragms at 90 to the stringers. diaphragms shall be spaced as shown in detail this sheet and as noted below.
3. Where aforementioned angle is greater than 70°, the diaphragms shall be parallel to the center line of bearing of the stringers.
4. Space intermediate diaphragms at 20'± to 25'± $\frac{1}{2}$; i.e. for spans. (Non-curved bridges only). Up to 25' $\frac{1}{2}$ bearings-no intermediate Diaphragm.
From 25' to 50' $\frac{1}{2}$ bearings-One Intermediate Diaphragm.
From 50' to 75' $\frac{1}{2}$ bearings-Two Intermediate Diaphragms, etc.
(See Framing Plan).
5. All diaphragms are to be completely connected to stringers before deck slab is poured.



DIAPHRAGM SPACING
70° OR LESS SKEW
Scale: None

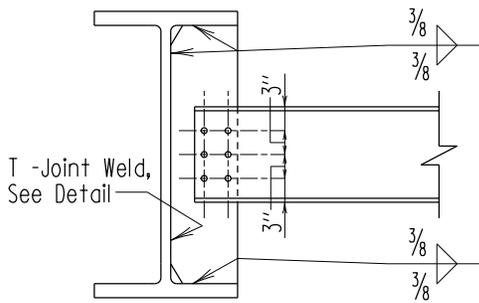


SKEW ANGLE
Scale: None

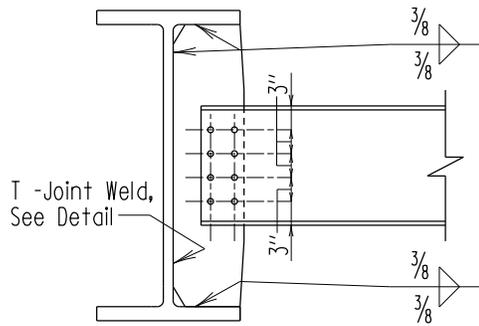
APPROVAL
<i>E.S. Friedman</i> DIRECTOR OFFICE OF STRUCTURES
DATE: 01/22/2003
VERSION
1.0

STATE OF MARYLAND DEPARTMENT OF TRANSPORTATION STATE HIGHWAY ADMINISTRATION OFFICE OF STRUCTURES
ROLLED STEEL BEAMS INTERMEDIATE DIAPHRAGM DETAILS WELDED CONNECTIONS
DETAIL NO. SUP-SS(CF)-101
SHEET 1 OF 2

SUPERSTRUCTURE - STEEL



24" & SMALLER
STRINGERS

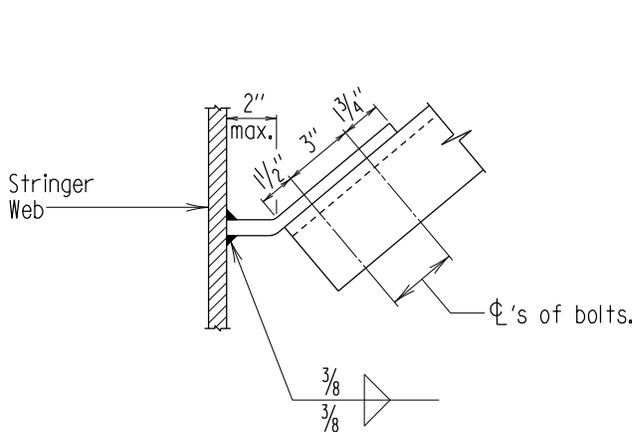


27" & LARGER
STRINGERS

See View "A" Below.

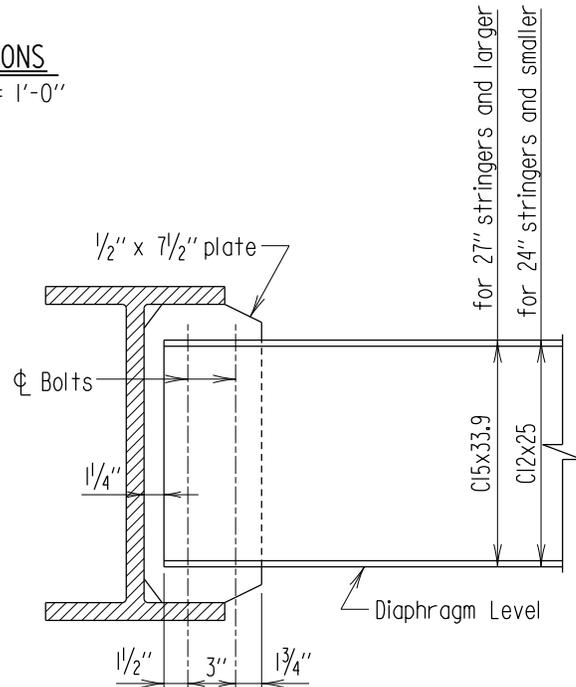
Note:
Angle clip shown;
optional radius clip
acceptable.

ELEVATIONS
Scale: 1/2" = 1'-0"



T-JOINT WELD DETAIL FOR
SKEW ANGLE OVER 70° TO 90°

Scale: None



Note: Dimensions shown are for 90° connections.

VIEW 'A'

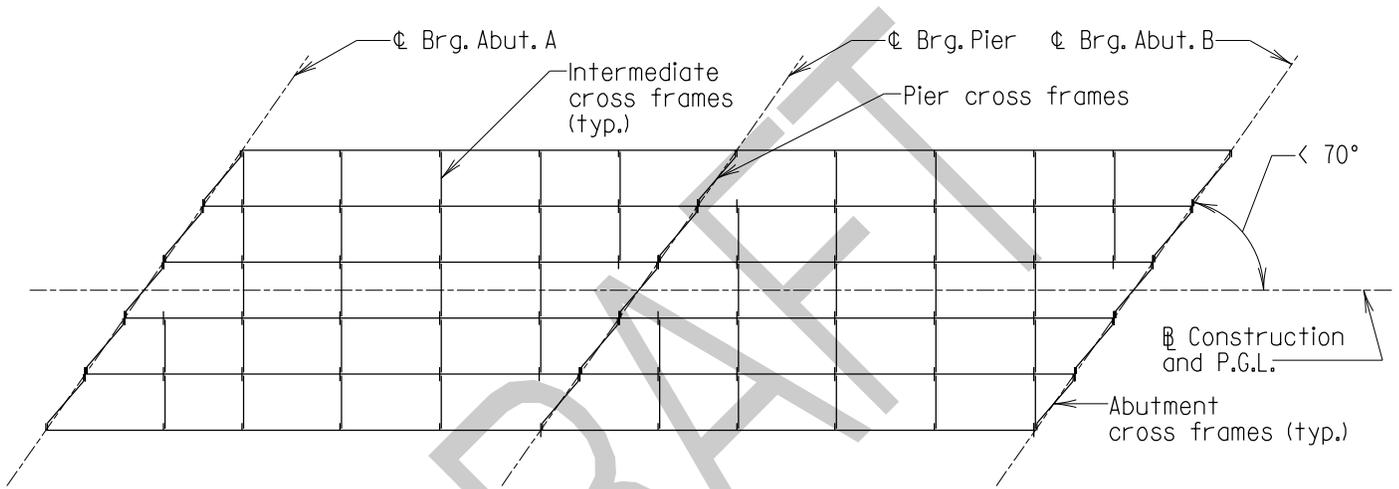
Scale: None

Notes:

1. For notes and all details not shown see sheet 1 of 2.
2. Contractor has option to use either welded or bolted connection. However only one type of connection may be used per bridge.
3. All bolts to be 1/8" ASTM A325.
4. All bolts holes to be 15/16" .
5. Bolt spacing applies regardless of skew.

APPROVAL
<i>L.S. Freeman</i> DIRECTOR OFFICE OF STRUCTURES
DATE: 10/22/2003
VERSION
1.0

STATE OF MARYLAND DEPARTMENT OF TRANSPORTATION STATE HIGHWAY ADMINISTRATION OFFICE OF STRUCTURES	
ROLLED STEEL BEAMS INTERMEDIATE DIAPHRAGM DETAILS BOLTED CONNECTIONS	
DETAIL NO. SUP-SS(CF)-101	SHEET 2 OF 2



FRAMING PLAN

Scale: None

Note:
 All cross frames are to be placed normal to girders or to the girder tangent and shall be continuous lines.

Note to Designer:
 The skew angle shown is the compliment of the skew angle as defined in AASHTO Section 6:7.4.

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APPROVAL
_____ DIRECTOR OFFICE OF STRUCTURES
DATE:
VERSION
DRAFT

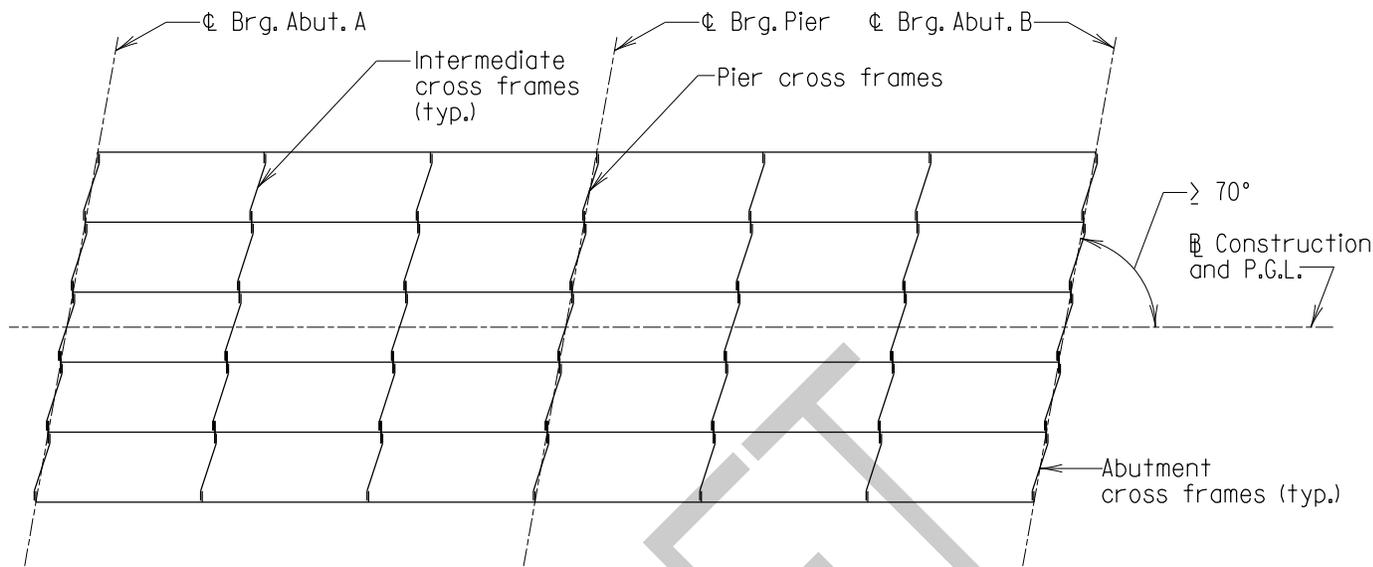
STATE OF MARYLAND
 DEPARTMENT OF TRANSPORTATION
 STATE HIGHWAY ADMINISTRATION
 OFFICE OF STRUCTURES

CROSS FRAM LAYOUT
 SKEW ANGLE IS LESS THAN 70°

DETAIL NO. SUP-SS(CF)-201

SHEET 1 OF 1

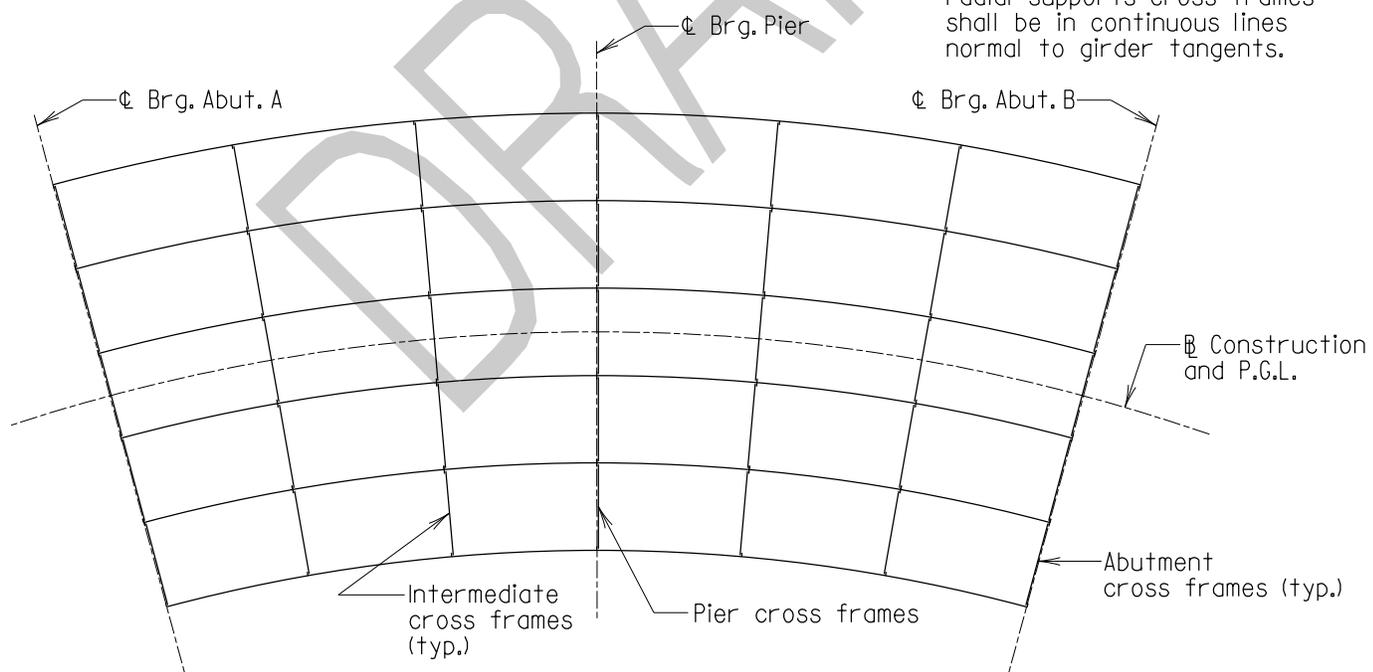
SUPERSTRUCTURE - STEEL



TANGENT FRAMING PLAN

Scale: None

Note:
 All cross frames are to be placed parallel to abutments and pier in a continuous line.
 For curved girders with radial supports cross frames shall be in continuous lines normal to girder tangents.



CURVED FRAMING PLAN

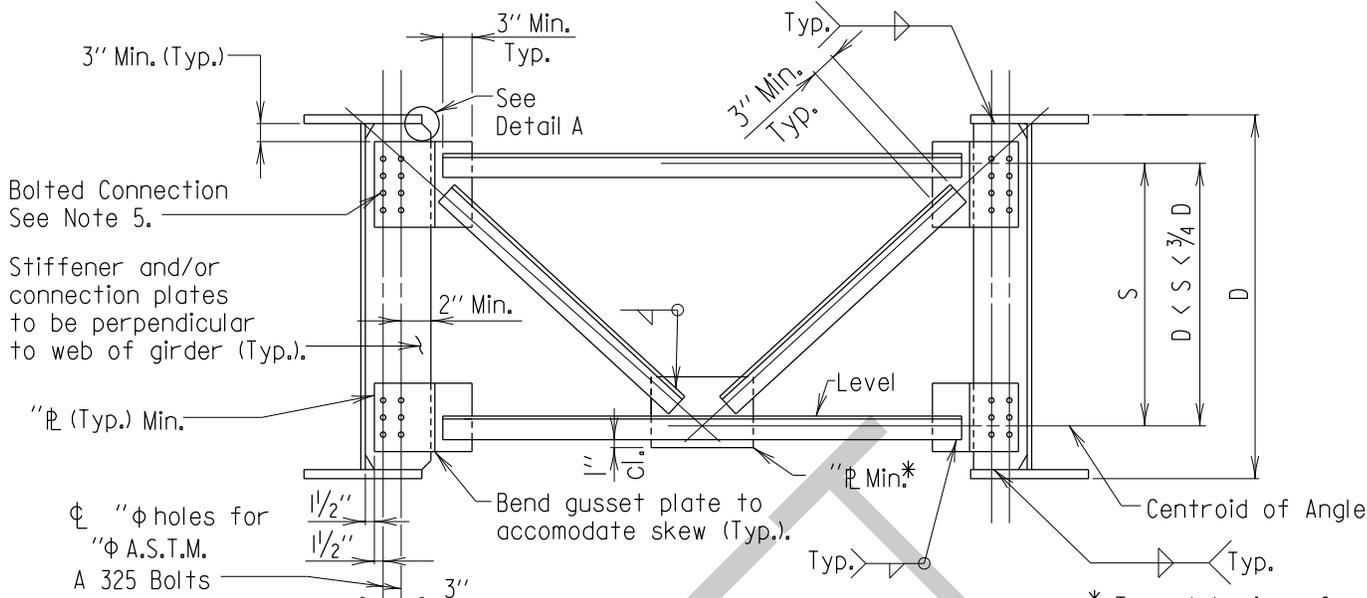
Scale: None

Note to Designer:
 The skew angle shown is the compliment of the skew angle as defined in AASHTO Section 6:7.4.

FOR OFFICE USE ONLY

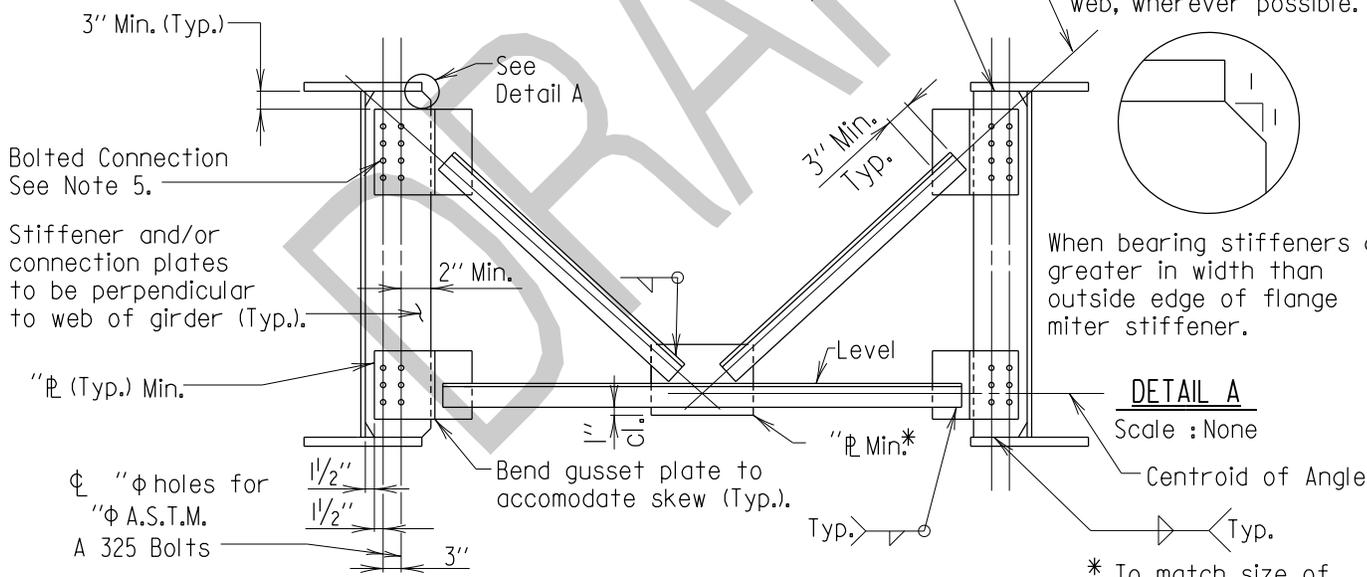
APPROVAL _____ DIRECTOR OFFICE OF STRUCTURES DATE: _____ VERSION _____ DRAFT	STATE OF MARYLAND DEPARTMENT OF TRANSPORTATION STATE HIGHWAY ADMINISTRATION OFFICE OF STRUCTURES CROSS FRAM LAYOUT SKREW ANGLE IS GREATER THAN OR EQUAL TO 70° OR RADIAL SUPPORTS
DETAIL NO. SUP-SS(CF)-202	SHEET <u> </u> OF <u> </u>

SUPERSTRUCTURE - STEEL



CURVED GIRDERS

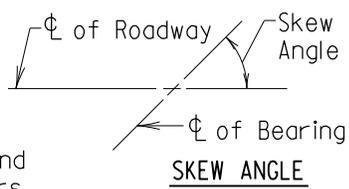
Scale: None



STRAIGHT GIRDERS

Scale: None

- Notes:
- Intermediate cross frames on curved girders shall be designed as main members.
 - Intermediate cross frames shall be placed as follows:
 - Curved Girders: Cross Frames shall be placed radially; and shall extend to intersection of ϕ bearing and ϕ girder at supports; wherever possible. Additional diaphragms shall be provided at all support areas.
 - Straight Girders: Where the skew angle between the center line of roadway and center line of bearing is 70° or less, place cross frames at 90° to the girders. Where aforementioned angle is greater than 70° , the cross frames shall be parallel to the center line of bearing of the girders.
 - Maximum interior cross frame spacing shall be 25 feet.
 - The smallest angle used for bracing shall be $3\frac{1}{2} \times 3\frac{1}{2} \times \frac{3}{8}$ inches.
 - Weld sizes and number of $\frac{7}{8}$ " ASTM A 325 bolts shall be determined by the Designer. Minimum size weld is $\frac{5}{16}$ in.
 - All cross frames are to be completely bolted and torqued before deck slab is poured.



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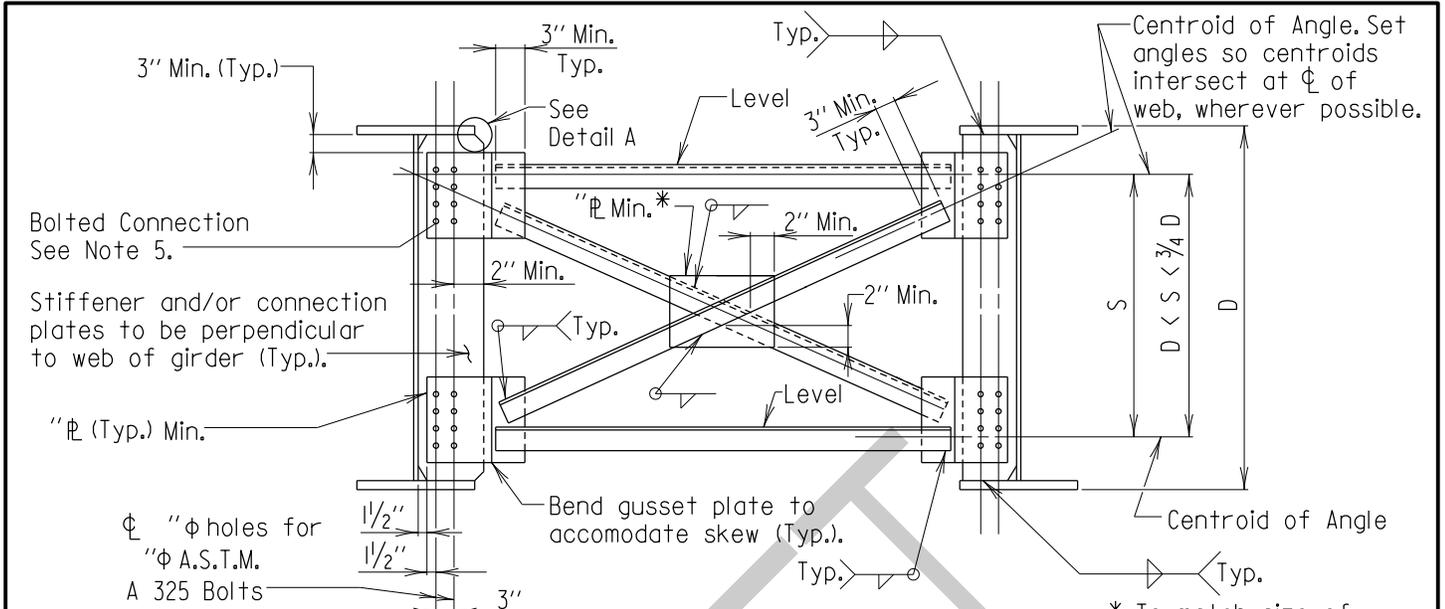
STATE OF MARYLAND
 DEPARTMENT OF TRANSPORTATION
 STATE HIGHWAY ADMINISTRATION
 OFFICE OF STRUCTURES

**INTERMEDIATE CROSS FRAMS (K-FRAMES)
 FOR FABRICATED STEEL GIRDERS**

DETAIL NO. SUP-SS(CF)-301

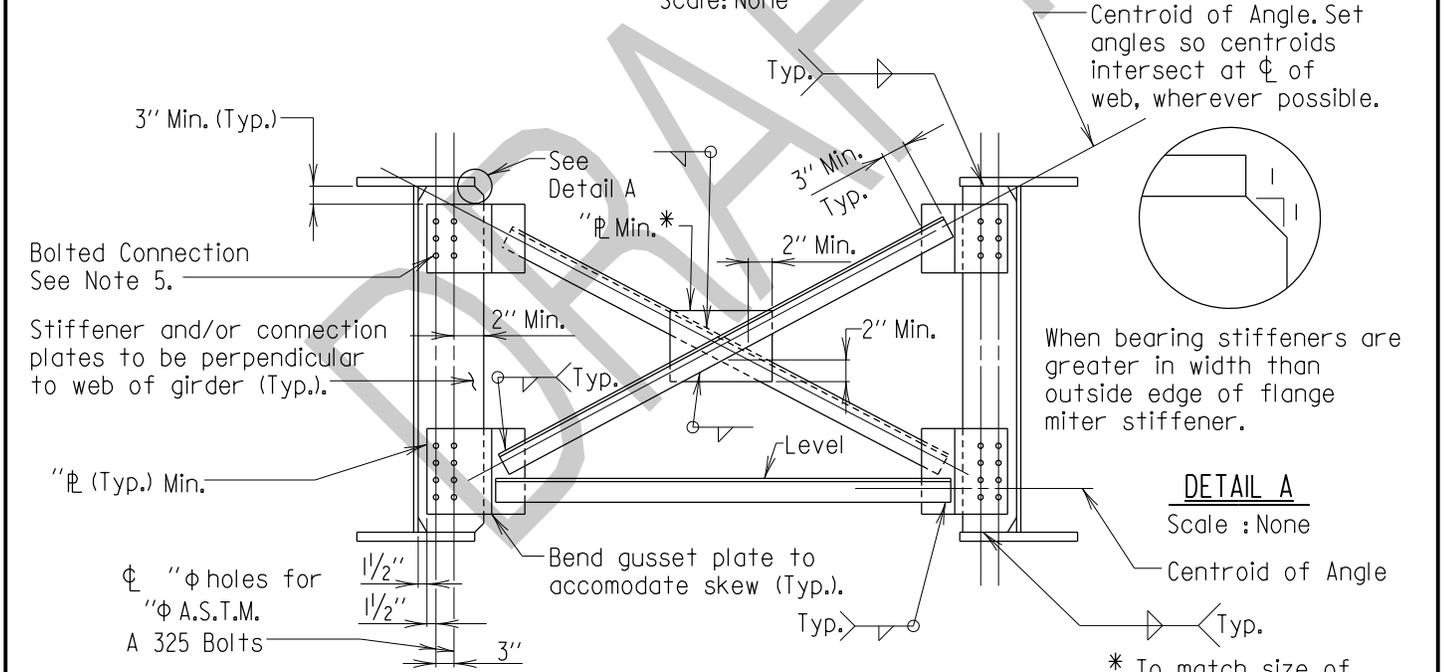
SHEET OF

SUPERSTRUCTURE - STEEL



CURVED GIRDS

Scale: None

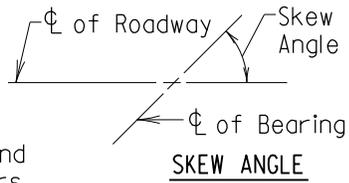


STRAIGHT GIRDS

Scale: None

Notes:

1. Intermediate cross frames on curved girders shall be designed as main members.
2. Intermediate cross frames shall be placed as follows:
 - Curved Girders: Cross Frames shall be placed radially; and shall extend to intersection of ϕ bearing and ϕ girder at supports; wherever possible. Additional diaphragms shall be provided at all support areas.
 - Straight Girders: Where the skew angle between the center line of roadway and center line of bearing is 70° or less, place cross frames at 90° to the girders. Where aforementioned angle is greater than 70°, the cross frames shall be parallel to the center line of bearing of the girders.
3. Maximum interior cross frame spacing shall be 25 feet.
4. The smallest angle used for bracing shall be 3 1/2 x 3 1/2 x 3/8 inches.
5. Weld sizes and number of 7/8" ASTM A 325 bolts shall be determined by the Designer. Minimum size weld is 5/16 in.
6. All cross frames are to be completely bolted and torqued before deck slab is poured.



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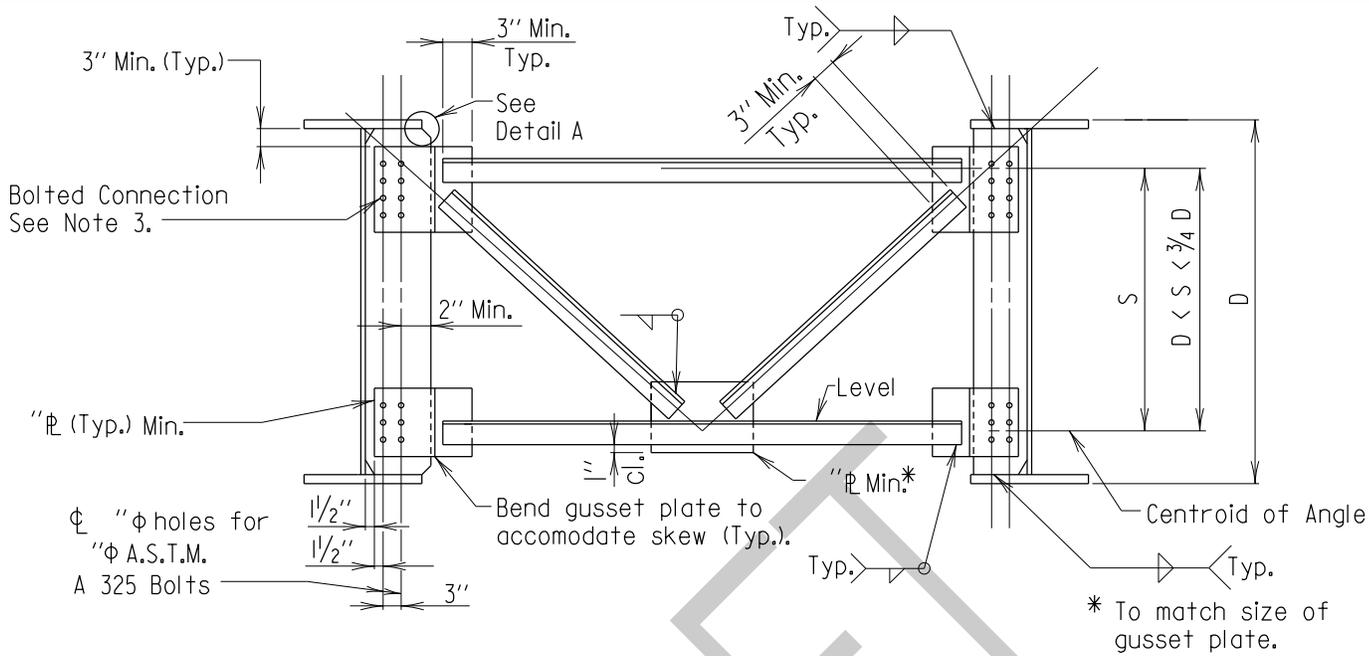
STATE OF MARYLAND
DEPARTMENT OF TRANSPORTATION
STATE HIGHWAY ADMINISTRATION
OFFICE OF STRUCTURES

**INTERMEDIATE CORSS FRAMES (X-FRAMES)
FOR FABRICATED STEEL GIRDS**

DETAIL NO. SUP-SS(CF)-302

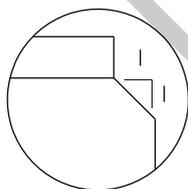
SHEET OF

SUPERSTRUCTURE - STEEL



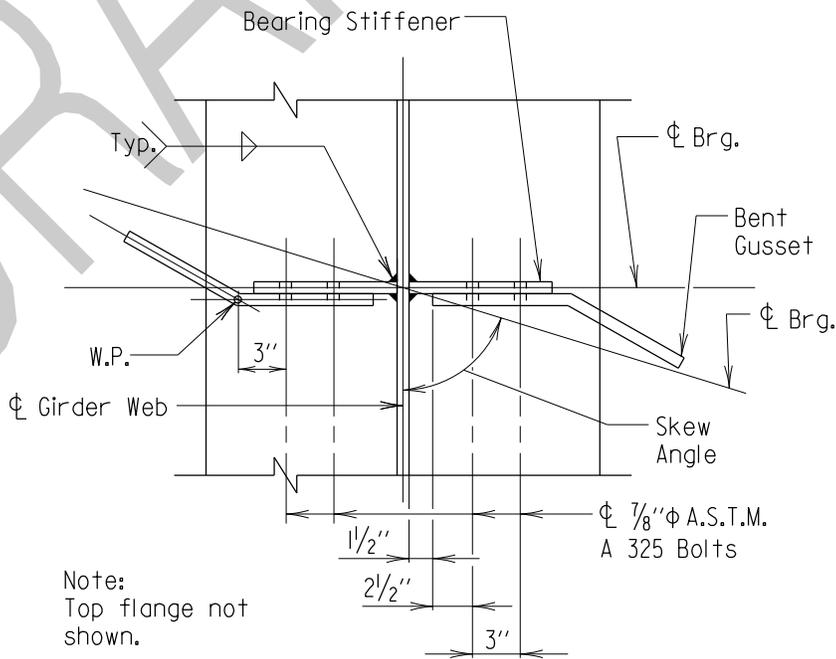
CURVED AND STRAIGHT GIRDERS

Scale: None



When bearing stiffeners are greater in width than outside edge of flange miter stiffener.

DETAIL A
Scale : None



CONNECTION DETAIL AT SKEWED PIER

Scale : None

Notes:

1. The smallest angle used for bracing shall be $3\frac{1}{2} \times 3\frac{1}{2} \times \frac{3}{8}$ inches.
2. Weld sizes and number of ASTM A 325 bolts shall be determined by the Designer. Minimum size weld is $\frac{5}{16}$ in.
3. All cross frames shall be completely bolted and torqued before deck slab is poured.
4. For bearing stiffener attachment see SUP-SS(GEN)-201.

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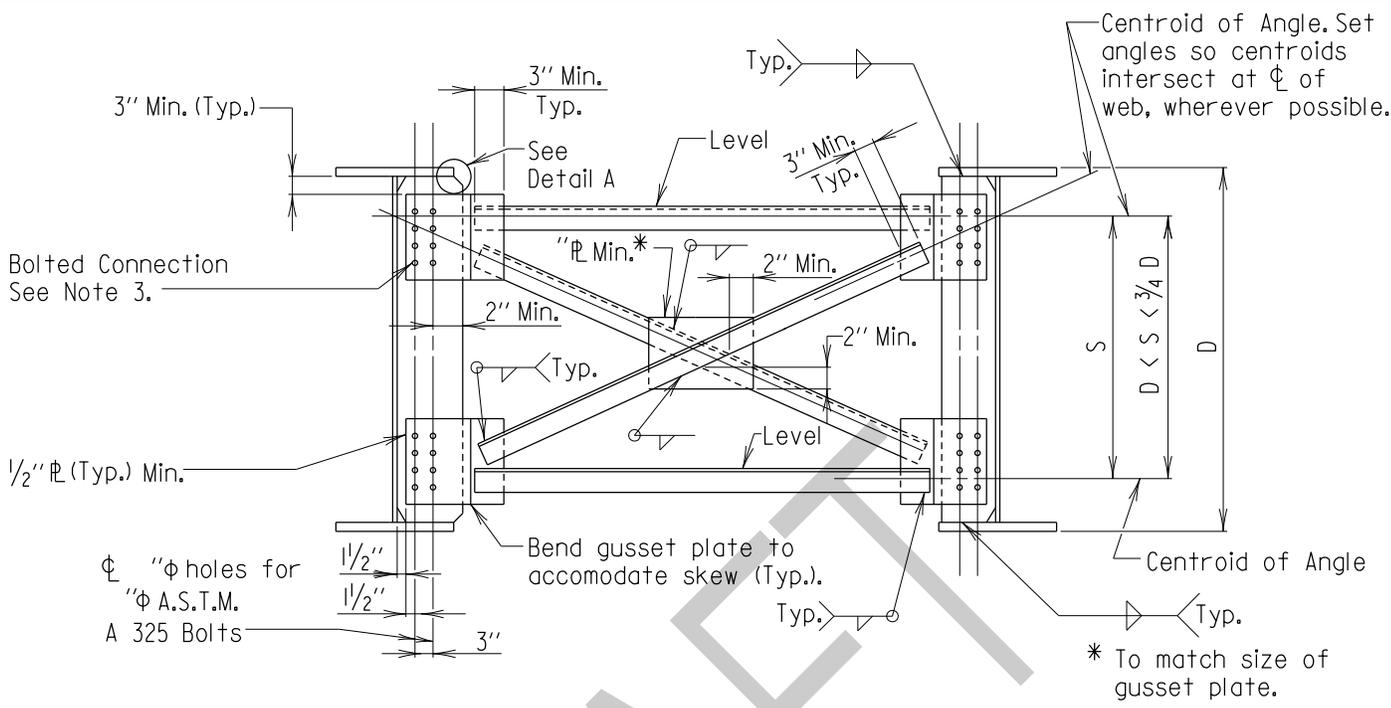
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DEPARTMENT OF TRANSPORTATION
STATE HIGHWAY ADMINISTRATION
OFFICE OF STRUCTURES

CROSS FRAMES (K-FRAMES) AT PIERS WHERE FABRICATED STEEL GIRDERS ARE CONTINUOUS

DETAIL NO. SUP-SS(CF)-401

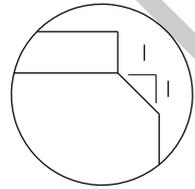
SHEET 1 OF 1

SUPERSTRUCTURE - STEEL



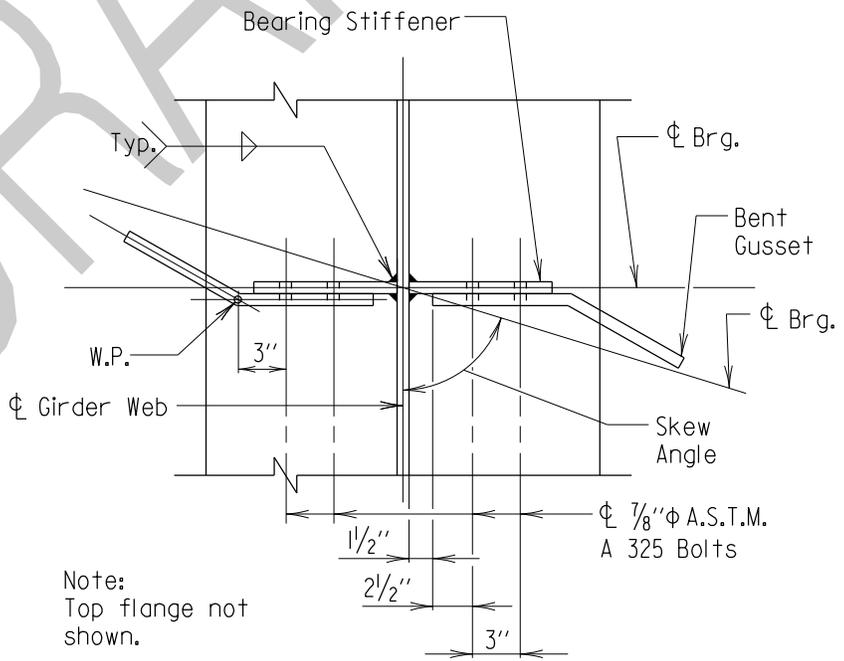
CURVED AND STRAIGHT GIRDERS

Scale: None



When bearing stiffeners are greater in width than outside edge of flange miter stiffener.

DETAIL A
Scale : None



CONNECTION DETAIL
AT SKEWED PIER

Scale : None

Notes:

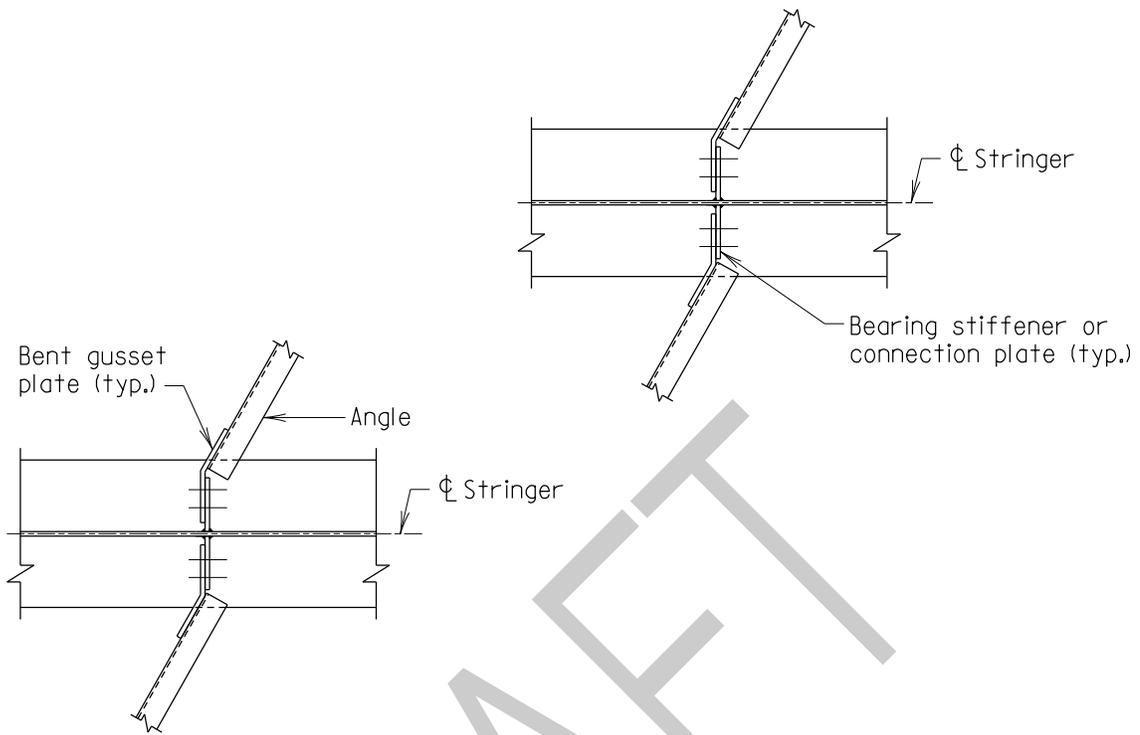
1. The smallest angle used for bracing shall be 3 1/2 x 3 1/2 x 3/8 inches.
2. Weld sizes and number of ASTM A 325 bolts shall be determined by the Designer. Minimum size weld is 5/16 in.
3. All cross frames shall be completely bolted and torqued before deck slab is poured.
4. For bearing stiffener attachment see SUP-SS(GEN)-201.

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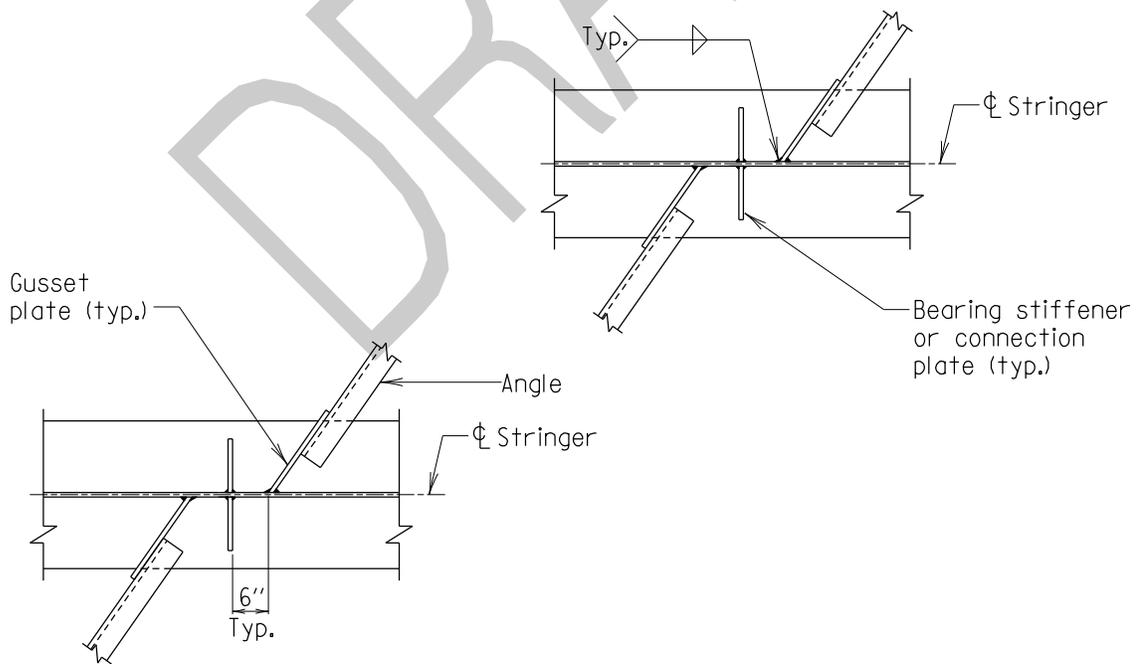
STATE OF MARYLAND DEPARTMENT OF TRANSPORTATION STATE HIGHWAY ADMINISTRATION OFFICE OF STRUCTURES	
CROSS FRAMES (X-FRAMES) AT PIERS WHERE FABRICATED STEEL GIRDERS ARE CONTINUOUS	
DETAIL NO. SUP-SS(CF)-402	SHEET <u> </u> OF <u> </u>

SUPERSTRUCTURE - STEEL



PLAN - 60° TO 90° SKEW ANGLES

Scale : $\frac{3}{8}'' = 1'-0''$



PLAN - 60° OR LESS SKEW ANGLE

Scale : $\frac{3}{8}'' = 1'-0''$

Notes:

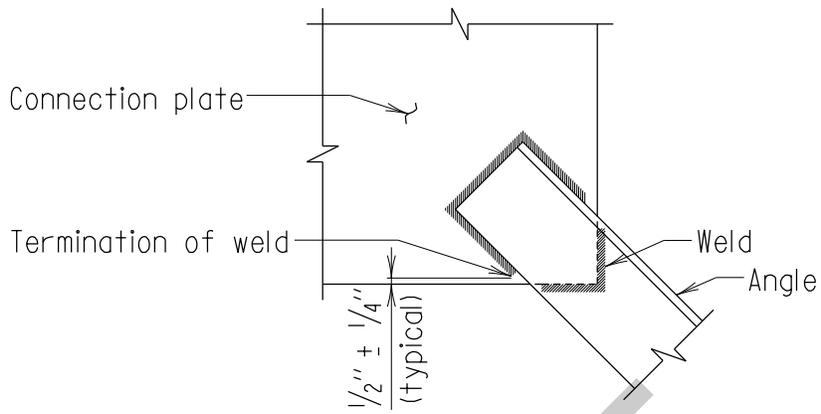
1. The smallest angle used for bracing shall be $3\frac{1}{2} \times 3\frac{1}{2} \times \frac{3}{8}$ inches.
2. Weld sizes and number of ASTM A 325 bolts shall be determined by the Designer. Minimum size weld is $\frac{5}{16}$ in.
3. All cross frames shall be completely bolted and torqued before deck slab is poured.
4. For bearing stiffener attachment see SUP-SS(GEN)-201.

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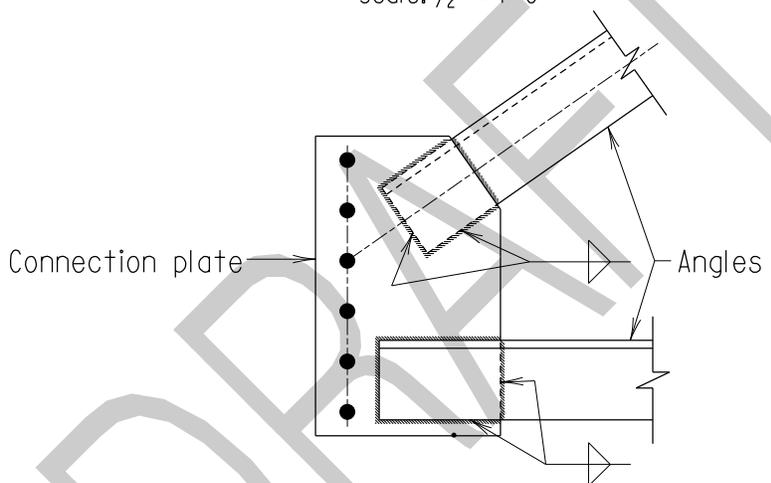
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STATE OF MARYLAND DEPARTMENT OF TRANSPORTATION STATE HIGHWAY ADMINISTRATION OFFICE OF STRUCTURES	
CROSS FRAM CONNECTIONS	
DETAIL NO. SUP-SS(CF)-501	SHEET <u> </u> OF <u> </u>

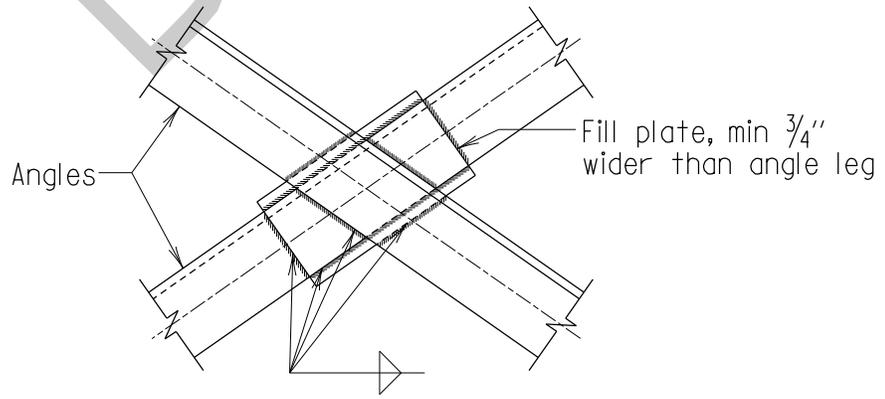
SUPERSTRUCTURE - STEEL



CROSS FRAME WELDING DETAIL
Scale: $\frac{1}{2}'' = 1'-0''$



DETAIL A
Scale: $\frac{1}{2}'' = 1'-0''$



DETAIL B
Scale: $\frac{1}{2}'' = 1'-0''$

Note:
All cross frame welds to be terminated from edge of gusset plates, fill plates or angles as indicated.

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STATE OF MARYLAND DEPARTMENT OF TRANSPORTATION STATE HIGHWAY ADMINISTRATION OFFICE OF STRUCTURES	
WELD TERMINATION DETAILS FOR CORSS FRAMES	
DETAIL NO. SUP-SS(CF)-502	SHEET <u> </u> OF <u> </u>

SUPERSTRUCTURE - STEEL

Chapter 03 - Superstructure

Section 05 – Structural Steel

SUB-SECTION 03

**DECK
REPLACEMENTS
(SUP-SS(DR))**

Welded
3/4" φ x 2" long
threaded stud.

Note:
Nut and washer
not shown.

15/16" vertical slot in
clip angle for 3/4" φ stud bolt.

Vertical leg of
joint angle.

Burn off vertical leg of existing clip
angle on this line and grind flush.
If remaining leg of angle does not
provide a full bearing surface for
new clip angle then completely
remove existing clip angle and
grind top flange to provide a proper
surface to receive new weld and
clip angle.

Burn off angle
leg if required,
to allow for
continuous weld.

After dam has been set and
bolted into final position, weld
clip angle to joint angle with 3/8"
continuous fillet weld.

Vertical leg of
joint angle.

SECTION A-A

1/2" Clip angle
size as
required.

φ Stringer

4" Min.

Vertical leg of
joint angle.

After dam has been set and
clamped into final position, weld
clip angle to beam with 3/8"
continuous fillet weld.

Existing Stringer

Note:
Studs not shown in PLAN.

PLAN

Scale: 1/2" = 1'-0"

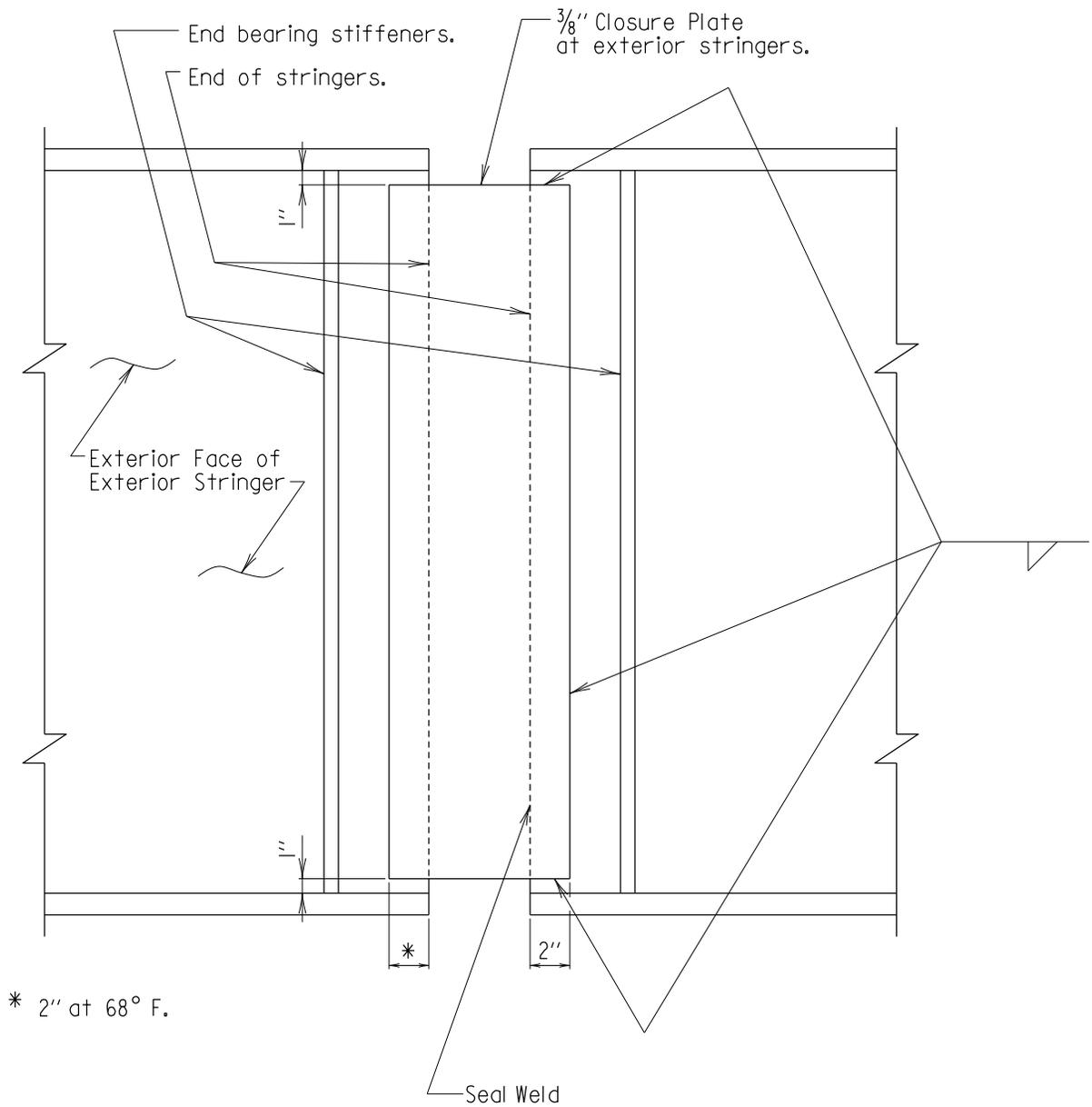
* If existing clip angle is exactly this
dimension or less, then new clip
angle along this edge is to be
beveled at contact surface so
that a proper weld can be provided.

Note:
Existing members shown dashed.

APPROVAL
<i>E.S. Freeman</i> DIRECTOR OFFICE OF STRUCTURES
DATE: 07/24/2001
VERSION
1.0

STATE OF MARYLAND DEPARTMENT OF TRANSPORTATION STATE HIGHWAY ADMINISTRATION OFFICE OF STRUCTURES	
ROADWAY JOINT - CLIP ANGLE DETAIL DECK REPLACEMENT - EXISTING STRINGER	
DETAIL NO. SUP-SS(DR)-101	SHEET <u>1</u> OF <u>1</u>

SUPERSTRUCTURE-STEEL



* 2" at 68° F.

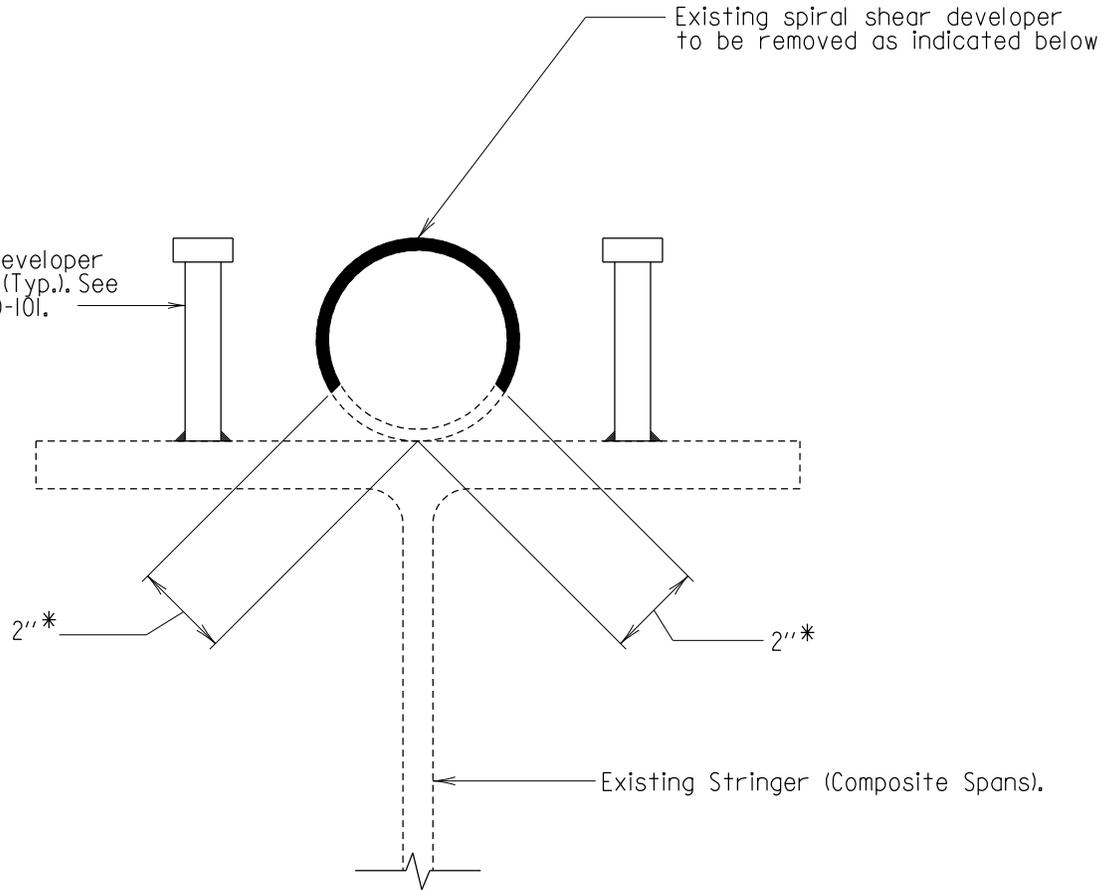
ELEVATION
Scale: None

- Notes:
1. Closure plates to be used on all exterior stringers at supports where stringers are not continuous.
 2. If stringers are of different depths, at a support, control dimensions shall apply to shallower stringer.
 3. Weld to stringer on fixed shoes, if possible, but only weld to one stringer.
 4. Do not provide closure plates on the median side of dual bridges where facias are 50' or less apart.

APPROVAL
<i>L.S. Friedman</i> DIRECTOR OFFICE OF STRUCTURES
DATE: 06/03/1976
VERSION
1.0

STATE OF MARYLAND DEPARTMENT OF TRANSPORTATION STATE HIGHWAY ADMINISTRATION OFFICE OF STRUCTURES
FASCIA STRINGER CLOSURE PLATE DETAIL
DETAIL NO. SUP-SS(DR)-201
SHEET <u>1</u> OF <u>1</u>

SUPERSTRUCTURE STEEL



SPIRAL REMOVAL

Scale: 3" = 1'-0"

* Maximum portion of existing spiral to remain.

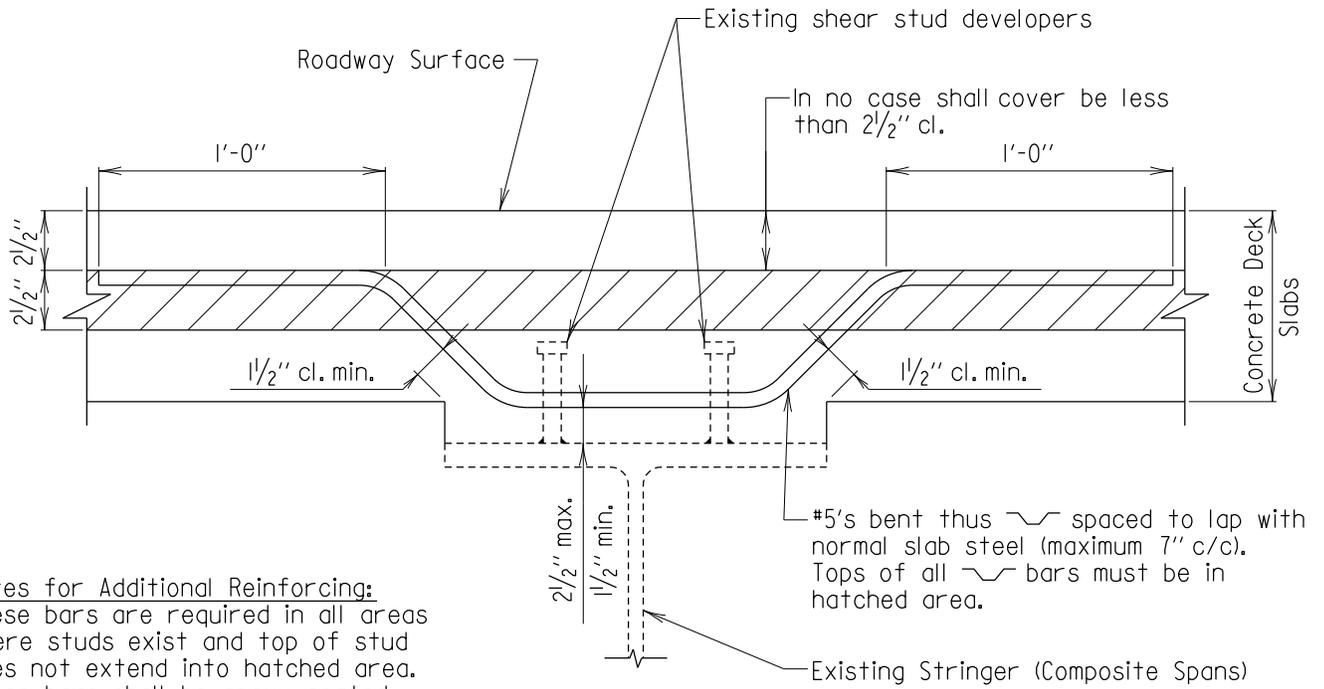
Notes for Spiral Removal and New Shear Studs:

1. Contractor shall cut all spiral shear developers off as close to flange as possible. Burning off spirals from flange will not be allowed.
2. For size and number of new studs per row and for longitudinal spacing of rows, see pertinent superstructure sheet.

APPROVAL
<i>L.S. Friedman</i> DIRECTOR OFFICE OF STRUCTURES
DATE: 05/19/2011
VERSION
1.0

STATE OF MARYLAND DEPARTMENT OF TRANSPORTATION STATE HIGHWAY ADMINISTRATION OFFICE OF STRUCTURES
DETAIL OF EXISTING SPIRAL REMOVAL
DETAIL NO. SUP-SS(DR)-301
SHEET <u>1</u> OF <u>1</u>

SUPERSTRUCTURE STEEL



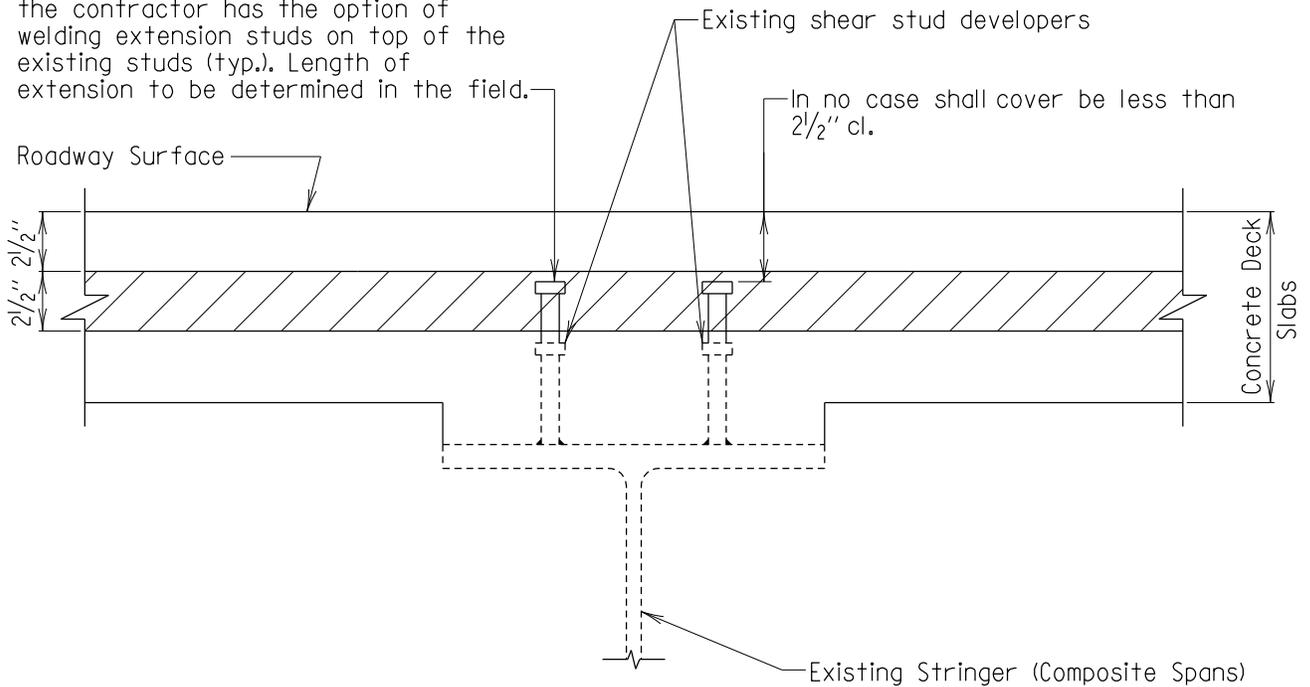
Notes for Additional Reinforcing:

1. These bars are required in all areas where studs exist and top of stud does not extend into hatched area.
2. These bars shall be epoxy coated.

SECTION

Scale: 1 1/2" = 1'-0"

In lieu of additional reinforcing steel, the contractor has the option of welding extension studs on top of the existing studs (typ.). Length of extension to be determined in the field.



SECTION

Scale: 1 1/2" = 1'-0"

Notes for Optional Extension Studs:

1. Extension studs are required in all areas where studs exist and top of stud does not extend into hatched area.
2. The diameter of the extension studs shall be the same as the existing studs.

APPROVAL
<i>E.S. Friedman</i> DIRECTOR OFFICE OF STRUCTURES
DATE: 05/19/2011
VERSION
1.0

STATE OF MARYLAND DEPARTMENT OF TRANSPORTATION STATE HIGHWAY ADMINISTRATION OFFICE OF STRUCTURES	
DETAIL OF ADDITIONAL REINFORCEMENT AT EXISTING SHEAR STUD DEVELOPERS OR ADDITION OF STUD EXTENSIONS	
DETAIL NO. SUP-SS(DR)-302	SHEET <u>1</u> OF <u>1</u>